

Engineer's/Drainage Report

for

LeRoy Taco Bell

Town/Village of LeRoy, Genesee County, New York

Prepared for:

Indus Hospitality Group
950 Panorama Trail South
Rochester, NY 14625

Prepared by:



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Fairport, NY 14450
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September 2026

BME Project #3036

Table of Contents

Figure 01 – Location Map

1.	Introduction.....	1
2.	Water Supply System	1
3.	Sanitary Sewer System	2
4.	Stormwater Management	2
	Overview	2
	Methodology	3
	Existing Conditions.....	3
	Proposed Conditions.....	3
	Bioretention Areas	4
	Soils	5
	Summary.....	6
5.	Erosion & Sediment Control.....	7
6.	Existing Environmental Conditions	8
7.	Site Access / Traffic Analysis	9

Appendices

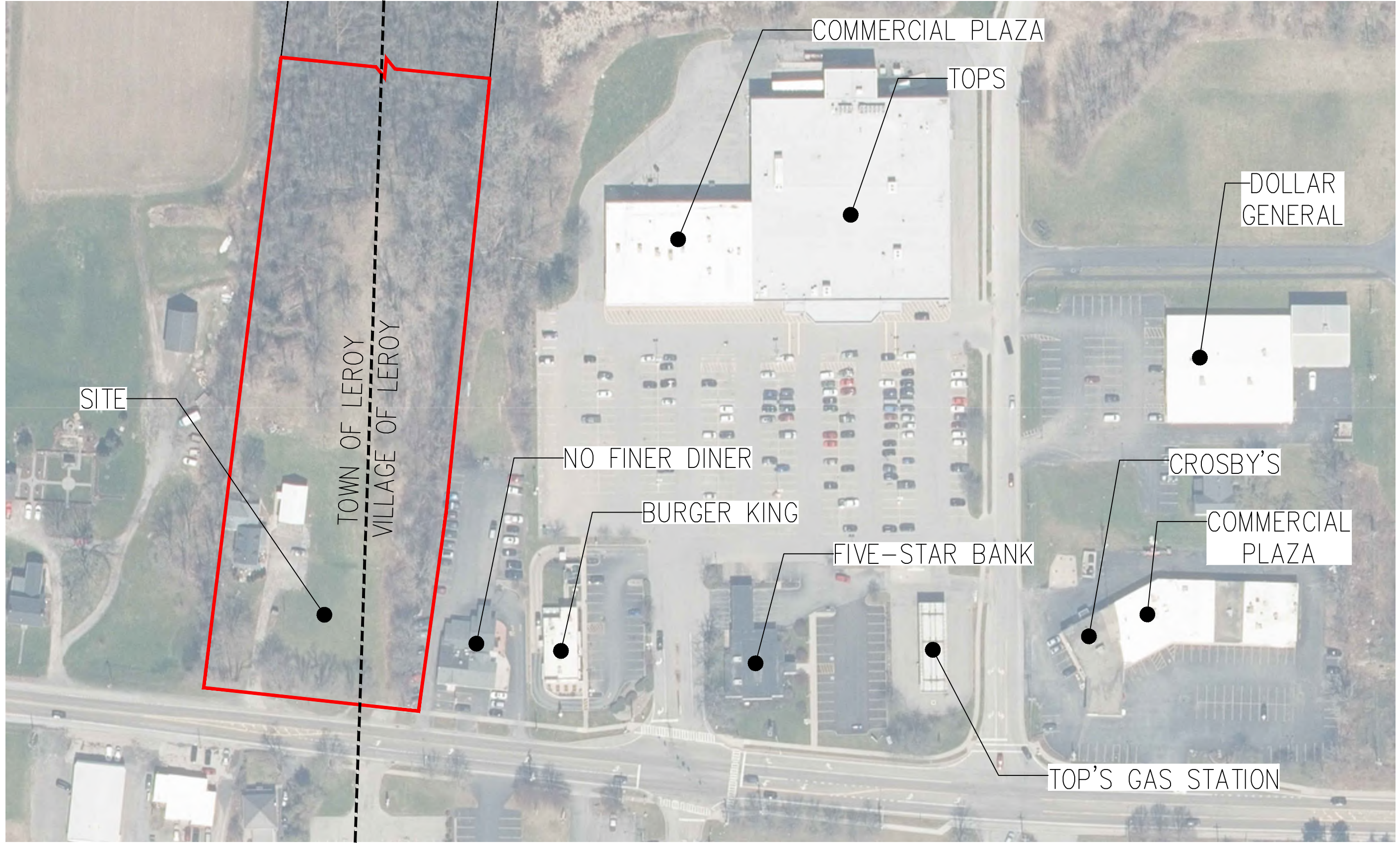
- A. Water Supply**
 - (1) Water Supply Calculations**
 - (2) Hydrant Flow Test Information**

- B. Stormwater Management**
 - (1) Figure 02 - Existing Drainage Map**
 - (2) Figure 03 - Proposed Drainage Map**
 - (3) Stormwater Hydrographs**
 - (4) USDA NRCS Web Soil Survey Soils Map**
 - (5) NYSDEC Bioretention Worksheet**
 - (6) Figure 04 – Storm Sewer Area Map**
 - (7) Storm Sewer and Rip-Rap Sizing**

- C. Background Information**
 - (1) NYS Cultural Resource Information System (CRIS) Map**
 - (2) NYS Wetland Mapper**
 - (3) BME Associates Wetland Delineation Map**
 - (4) SHPO Letter of No Impact**

- D. Vehicle Turning Analysis**
 - (1) Exhibit**
 - (2) NYSDOT Correspondence**

- E. Pump Station**



1) Introduction

Indus Hospitality Group (Indus) is proposing the construction of a $\pm 2,235$ sf Taco Bell drive-thru restaurant in the Town of LeRoy to be located at 7185 W Main St. The proposed location of the restaurant is under the jurisdiction of the Town of LeRoy. The project site is zoned C-2 General Commercial. The proposed use as a drive-through restaurant is allowed per Town code under a Special Use Permit.

The following narrative for this project provides the technical data to support the proposed action and includes information regarding water supply, sanitary sewer, stormwater, site access and trip generation calculations and existing environmental mapping.

2) Water Supply System

Water service to the proposed Taco Bell will be supplied via a proposed 1.5" PE domestic water service to be tapped off an existing ductile iron watermain located along the northern side of West Main Street. Per record maps, the water main is a 12" main. The watermain is within the NYSDOT's right-of-way and thus a NYSDOT Highway Work Permit will be required prior to any taps.

The proposed 1.5" service will provide sufficient water pressure and flow at the proposed building per the calculations provided in Appendix A. No sprinkler system and thus fire service is proposed nor required for the building per NYS Building Code or Local Code. The proposed 1.5" service will enter the building at the East side and will be backflow protected (RPZ) and metered just inside of the building. A public hydrant is located on the northern side of West Main Street, south of the property, which is less than 300' from the furthest corner of the proposed building, meeting the requirements for hose length.

Appendix A includes the hydraulic calculations under domestic flow conditions for the proposed building. Analysis of the proposed conditions show that the proposed building demands meet the ISO-recommended pressure minimums of 35 psi under domestic conditions. The estimated average daily domestic demand for the proposed restaurant is $\pm 1,500$ gpd. This is based on 25 gpd per seat (40 seats total) and 500 gpd per drive-up window. The system is capable of providing a domestic flow of 50 gpm at 51 psi. Hydrant flow data used in the water supply calculations was supplied by the Monroe County Water Authority on 8/6/26 and is included in Appendix A. These calculations will be submitted to the Monroe County Water Authority for their review and ultimate approval of the water service tap.

3) Sanitary Sewer / Private Pump Station

The proposed development will be served by a proposed private ±300' 1.5" HDPE DR-11 sanitary forcemain from a proposed on-site Liberty D3600 (or approved equivalent) private pump sanitary duplex grinder pump station with HDPE tank and alarm panel, north of the proposed Taco Bell building. The private forcemain will connect to the Village of LeRoy's existing public 8" sanitary sewer main that runs Easterly, southeast of the site. The private forcemain will connect to an existing manhole located approximately 40' east of the site and is within the NYSDOT's right-of-way, and thus will require a utility Highway Work Permit.

The contractor shall field verify depths and location of the existing sewer main for the tap and shall be constructed per the Village of LeRoy's requirements. Indus has submitted a sanitary sewer tap application to the Village and has received verbal consent. It is their intent to work with the Village leading up to the tap of the forcemain.

The estimated sewage flow from the proposed restaurant is ±1,500 gpd (25 gpd * 40 seats + 500 gpd for drive-up window) per the NYSDEC Design Standards for Wastewater Treatment Works manual recommendation for a food service operation. A separate grease lateral will be extended from the restaurant and routed through an exterior grease interceptor (1500 gallon min.) prior to combining with the sanitary lateral and connection to the pump station and forcemain. In conversations with the Village, there are no known capacity issues that exist today. Appendix E includes the Liberty Pump details.

4) Stormwater Management

Overview:

The proposed development is less than 1 acre of disturbance which includes utility trenching and grading work associated with the development, and therefore no post-construction stormwater controls are required per the current New York State Department of Environmental Conservation (NYSDEC) Stormwater Management Design Manual (SWMDM) for water quality and quantity requirements. Nor coverage needed from the NYSDEC SPDES General Permit for Stormwater Discharges from Construction Activity. The Town or Village of LeRoy also does not have specific stormwater requirements for stormwater management for projects less than 1 acre of disturbance. The project site is approximately 15.17 acres and 0.99 acres is proposed to be disturbed. However, in discussion with the Town and their consultants, the development has been evaluated to detain runoff on-site, and utilize stormwater bioretention areas.

Methodology:

The existing and proposed drainage conditions at the project site were analyzed following the methods outlined in Soil Conservation Service Technical Release No. 55. Peak runoff rates for existing and post-development conditions were modeled for the 1, 2, 5, 10, 25, 50, and 100-year 24-hour design storm events using the HydroCAD 10.10-7a 2021 software. Runoff rates were determined based on the hydrologic characteristics of the site (soil conditions, existing and proposed land cover, time of concentration for the contributing drainage areas).

Appendix B includes HydroCAD calculations, which contain curve numbers, time of concentration calculations for each of the modeled subareas, and summary reports for each of the peak storm events. Appendix B also contains information regarding the soils located at the project site.

Existing Conditions:

The analyzed watershed is ± 1.13 acres which largely includes the previously developed portion of the site and the location of the proposed development disturbance for comparison purposes.

Under existing conditions, the site is mostly comprised of mowed lawn area, two existing homes exist on the property with impervious sidewalks and driveways. The site generally sheet flows to the east to the existing drainage ditch that runs north along the eastern property line (Subarea A), the banks of the existing drainage ditch was chosen as the Analysis Point A. A small portion of the site sheet flows north to undisturbed lands and ultimately the wetlands (Subarea B), and a small portion of the sites sheet flows west to the wooded area along the western property line (Subarea C). Appendix B includes the Existing Conditions Map.

Proposed Conditions:

The analyzed watershed is ± 1.13 acres. Under post-development conditions, existing drainage patterns will be maintained. Subarea A-1 and A-2 are comprised of the areas that will drain to the on-site bioretention areas. These areas consist of mostly impervious area, with some mowable lawn areas. The bioretention areas include 2'x2' storm outlet structures with outlet pipes that tie into the proposed on site storm sewer system. The storm sewer ultimately discharges to the banks of the existing drainage ditch at the northeast corner of the property. The discharge point was chosen as it is lowest point in elevation on site. The outlet will include stone rip rap for erosion control.

There are small areas that consists of all lawn and trees that will continue to sheet flow to the north and west. However, these areas are smaller in size than under existing conditions and thus the volume and

peak flow rate of stormwater runoff directed towards Analysis Point B and C are reduced from existing conditions.

Filtration Bioretention Areas:

Bioretention Area 1:

The proposed bioretention area is being construction within the eastern portion of the site, within the lawn area east of the Taco Bell building and parking lot/drive aisle and receives runoff from Subarea A-1, which is a majority of the project site and impervious area.

The bioretention area will provide water quality and extended detention on site. A stone diaphragm and rip rap have been provided where stormwater is allowed to sheet flow and discharge to the practice. To help remove first flush pollutants prior to being filtered through the bioretention media. The intent is to replicate pre-construction infiltration, peak runoff flow and discharge volume, as well as minimization of concentrated flow by using runoff control techniques prior to reaching the offsite discharge locations. The practice also has an underdrain system and includes an emergency outlet structure which is set above the bioretention media. The outlet structure would discharge to the proposed storm sewer system at a reduced rate prior to discharging to the drainage ditch to the east.

See Appendix B for NYSDEC sizing calculations. The facility has been designed to detain up to the 100-year design storm elevation. Table 1 below contains the ponding elevations and outflow data for the facility:

Table 1: Bioretention Area 1 Summary

Design Storm Event	Post-Dev. Inflow (cfs)	Post-Dev. Outflow (cfs)	Ponding Elevation (ft)	Storage Volume (cf)
1 Yr. / 24 Hr.	1.05	0.05	882.56	1,470
10 Yr. / 24 Hr.	1.81	0.99	883.05	1,760
100 Yr. / 24 Hr.	3.08	2.21	883.23	2,144
Top of Media Elev	883.0		Total Available Storage Volume (cf) = 2,828	
Outlet Structure TG Elev	883.5			

Bioretention Area 2:

The proposed bioretention area is being construction within the northern portion of the site, within the lawn area north of the Taco Bell building and receives runoff from Subarea A-2 which is the remaining impervious on site including the Taco Bell roof, and drive-thru lane.

The bioretention area will provide water quality and extended detention on site. A stone diaphragm and rip rap have been provided where stormwater is allowed to sheet flow and discharge to the practice. To help remove first flush pollutants prior to being filtered through the bioretention media. The intent is to replicate pre-construction infiltration, peak runoff flow and discharge volume, as well as minimization of concentrated flow by using runoff control techniques prior to reaching the offsite discharge locations. The practice also has an underdrain system and includes an emergency outlet structure which is set above the bioretention media. The outlet structure would discharge to the proposed storm sewer system at a reduced rate prior to discharging to the drainage ditch to the east.

See Appendix B for NYSDEC sizing calculations. The facility has been designed to detain up to the 100-year design storm elevation. Table 2 below contains the ponding elevations and outflow data for the facility:

Table 2: Bioretention Area 2 Summary

Design Storm Event	Post-Dev. Inflow (cfs)	Post-Dev. Outflow (cfs)	Ponding Elevation (ft)	Storage Volume (cf)
1 Yr. / 24 Hr.	0.42	0.02	880.64	517
10 Yr. / 24 Hr.	0.81	0.02	881.10	1,264
100 Yr. / 24 Hr.	1.47	0.05	883.56	2,490
Top of Media Elev	883.0		Total Available Storage Volume (cf) = 3,539	
Outlet Structure TG Elev	884.0			

Soils:

Enclosed in Appendix B of this report is a hydrologic soil group map from USDA NRCS Web Soil Survey (Figure 02). The mapped soils on the project site include Kendaia silt loam (B/D rating). A Geotechnical Evaluation was completed on the site in August 2026 by Foundation Design P.C. and observed the subsurface conditions to be typical of a redevelopment site including topsoil or asphalt, fill, glacial lake deposits, then glacial till underlaid by bedrock. Bedrock was encountered greater than 16'

deep and groundwater was encountered greater than 5' deep. Infiltration tests were completed on site to determine the underlying soils were not infiltrative and thus for the drainage analysis, a hydrologic class "D" was utilized.

Summary:

The NYSDEC Stormwater Design Manual establishes that these quantity controls are required to be reduced for redevelopment and new development projects. Post development peak runoff rates have been attenuated on site to be equal to or less than pre development rates for the 1-year storm (Channel Protection Volume), 10-year Storm (Overbank Flood Control) and the 100-year Storm (Extreme Flood Control). Table 3-6 below are Peak Flow Comparisons for the site and all demonstrate that under post development conditions, the quantity controls are met. Given that the total peak runoff rates are not exceeded with the proposed development, the proposed stormwater management plan effectively attenuates the post-development runoff for all storm events. See Appendix B for pre and post development drainage conditions as well as peak runoff rates that were modeled in HydroCAD.

Table 3: Comparison of Peak Runoff Rates (Total Site)

Analysis Point	Design Storm	Existing Peak Runoff Rate (cfs)	Proposed Peak Runoff Rate (cfs)
Off-Site (Combined A+B+C)	1-yr	0.71	0.21
	10-yr	1.81	1.42
	100-yr	3.89	3.17

Table 4: Comparison of Peak Runoff Rates (Analysis Point A)

Analysis Point	Design Storm	Existing Peak Runoff Rate (cfs)	Proposed Peak Runoff Rate (cfs)
Eastern Drainage Ditch Banks (Analysis Point A)	1-yr	0.63	0.20
	10-yr	1.57	1.35
	100-yr	3.33	3.01

Table 5: Comparison of Peak Runoff Rates (Analysis Point B)

Analysis Point	Design Storm	Existing Peak Runoff Rate (cfs)	Proposed Peak Runoff Rate (cfs)
Western Property Line (Analysis Point B)	1-yr	0.04	0.03
	10-yr	0.15	0.09
	100-yr	0.35	0.21

Table 6: Comparison of Peak Runoff Rates (Analysis Point C)

Analysis Point	Design Storm	Existing Peak Runoff Rate (cfs)	Proposed Peak Runoff Rate (cfs)
Northern wetland (Analysis Point C)	1-yr	0.05	0.01
	10-yr	0.13	0.02
	100-yr	0.28	0.05

Storm Sewer & Rip Rap: See Appendix B for storm sewer and stone rip rap sizing for the project site.

5) Erosion & Sediment Control

A Construction Erosion Control Plan (BME Dwg. #3036-06) has been prepared and includes inlet protection, silt socks, a concrete washout, soil stabilization methods, silt fence and a sequence of construction. The prepared plan has been designed per the New York Standards and Specifications for Erosion and Sediment Control. Erosion control measures listed will be implemented during construction to control silt and minimize disturbance to the existing drainage conditions. The details are per the NYS Standards and Specifications for Erosion and Sediment Control and are provided within the plan set. The site will include a stabilized construction access and will be monitored to minimize the tracking of sediment onto public streets. Disturbed areas will be re-established as soon as possible to prevent erosion during construction activity. The project will not be phased.

Special care will be taken to prevent the transportation of silt from disturbed areas to downstream areas. Controls in the form of filter fabric inlet protection and silt socks will be provided at appropriate inlets on and off site. Disturbed areas will be seeded and mulched as soon as possible to prevent erosion. The design plans will be reviewed at the pre-construction meeting with all involved parties. See the BME Landscape

Plan & Erosion Control plan for a description of the vegetative measures to be used for soil stabilization and runoff/sediment control including seeding mixtures and rates.

Maintenance is also a part of the construction erosion control plan. The developer's contractor is responsible for installing all specified erosion control features, as well as maintaining and replacing them as needed throughout construction. The developer will periodically review the erosion control measures to determine their efficiency and possible need for replacement. The owner's contractor, sub-contractors and all others associated with the implementation of the erosion control plan shall be familiar with the plan and the conditions of the Town as necessary.

6) Existing Environmental Conditions

The project site has been reviewed for occurrence of Federal and NY State regulated wetlands. BME Associates conducted a field investigation and wetland delineation of the site. The proposed development has been designed to avoid disturbance of all designated wetland areas. There are also no rare plants or animals, endangered or threatened species, or national natural landmarks located on the project site or adjacent to. The project site is not located within an Agricultural District. The Wetland Delineation Map is included in Appendix C for your use.

Per the NYS Department of Parks, Recreation and Historic Preservation (OPRHP) State Historic Preservation (SHPO) Cultural Information System (CRIS) online mapper, the project site is located within an archeological buffer area. The project site has been submitted to the State Historic Preservation Office for a Letter of No Impact which has been received and is included within Appendix C for your files. The Letter notes that a landscape buffer is to remain between the property and the western adjoiner. This condition is met as an approximate 10'-70' buffer resides on the western adjoining property and will not be removed as part of this development. Within the project disturbance there have been prior ground disturbance including public utilities, roads and landscaped areas. There are no hydrographic features located within the area disturbance. There are no Registered Historical places within the project disturbance. See Appendix C for applicable mapping.

Review of the FEMA maps indicates that no portion of this project was recognized to be in a 100-year floodplain and is designated Zone C in FEMA Floodplain Panel 360379 0001 C, effective date September 14, 1979. No designated floodway exists near the project site, and thus no fill is proposed within a floodway.

7) Site Access / Traffic Analysis

There is one new curb cuts proposed for the Taco Bell development site from West Main Street. NYSDOT highway work permits are anticipated for the access to West Main Street (New York State Route 5) and proposed concrete sidewalk connection to the existing W. Main St. sidewalk network.

The Taco Bell drive-thru lane has been designed to provide the minimum number of car stacking required by Taco Bell and account for peak times which allows this number of cars to stack on site and not back up into Village public streets. A parking lot for 21 spaces is provided adjacent to Taco Bell with mobile order designated spaces and ADA accessible parking spaces (as required per code). An enclosed refuse area has been shown on the site and included in Appendix D is a turning template exhibit for the site which demonstrates that vehicles that could visit the site would be able to maneuver appropriately.

The proposed trip generation for the restaurant has been established per the methods of the Institute of Transportation Engineers (ITE) Trip Generation Manual. Table 3 below indicates the total site generated trips for the AM and PM peak hours for the proposed use:

Table 3: Site Trip Generations

Land Use	ITE Group	Size	Seats	AM Peak Hour		PM Peak Hour	
				Enter	Exit	Enter	Exit
Fast Food Restaurant with Drive-Through	934	+/-2,235 sf	40	38	36	37	33
Pass-by Rates (30% AM, 40% PM) *				-11	-10	-11	-10
Total Primary Trips				27	26	27	23

*The service land use type is expected to generate pass-by trips. Pass-by rates were based on the ITE Trip Generation Handbook and were assigned as 30% during the weekday morning peak hour, and 40% during the weekday evening and Saturday midday peak hours.

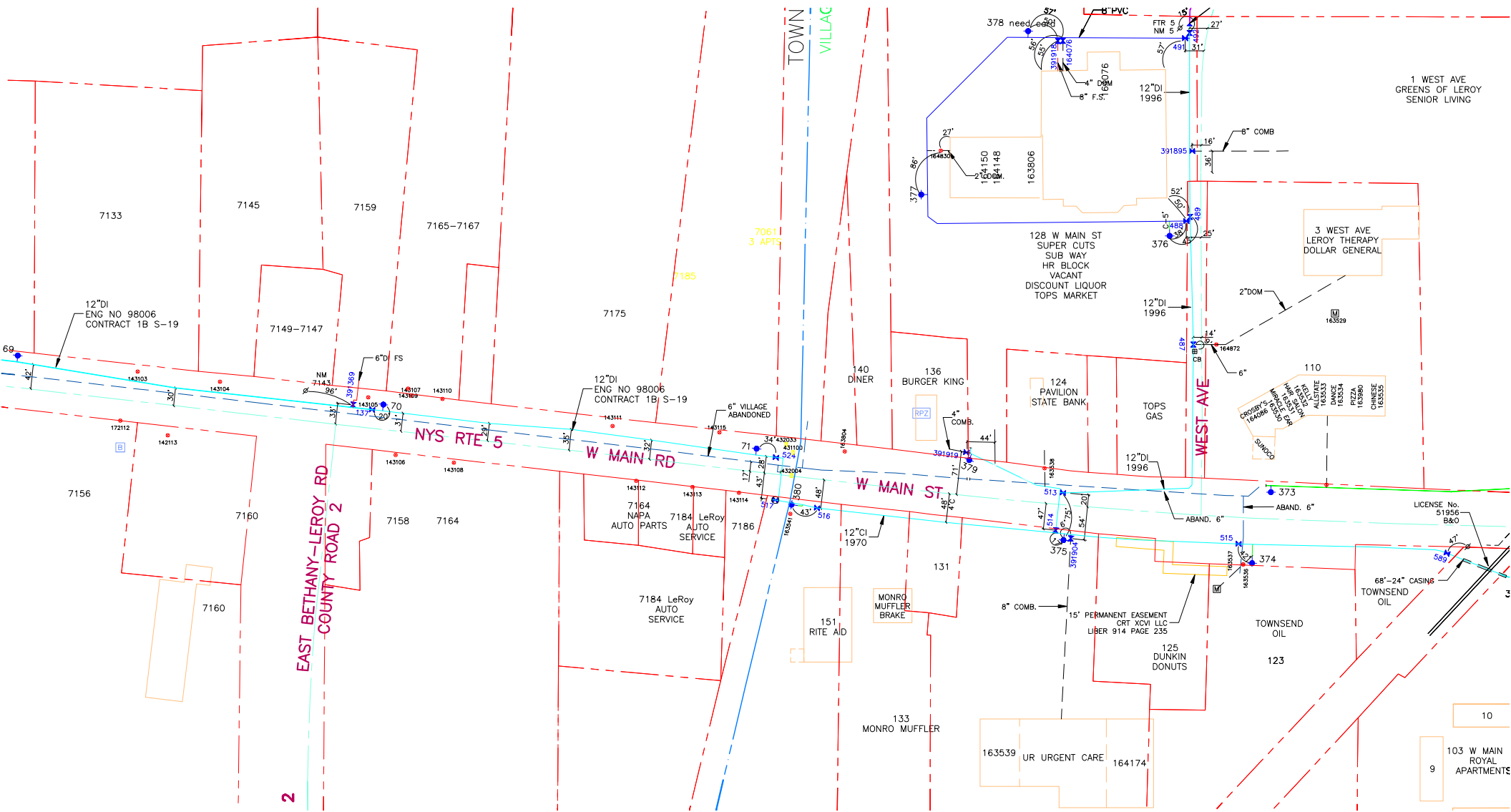
As noted, not all these trip generations are new, but instead a portion of the proposed volume is reduced considering pass-by credits. The proposed project is expected to generate approximately 27 entering / 26 exiting vehicle trips during the AM peak hour and 27 entering / 23 exiting vehicle trips during the PM peak hour. Based on the proposed generated trips it appears the proposed project will not result in any potentially significant adverse traffic impacts to the surrounding road network.

Correspondence with the NYSDOT is included within Appendix D of this report and indicates they have reviewed the proposed development, are in concurrence with the access to NYS Route 5 and have concluded

that no traffic analysis is required for this project prior to its occupation of the site. Typical Highway Work Permits for Utilities and widening the access drive will be required.

Appendix A

Water Supply Calculations



10

103 W MAIN
ROYAL
APARTMENTS

9

Computer Simulated Fire Flow Test/Supply Curve

Location: W Main Rd & E Bethany LeRoy Rd

Town of LeRoy

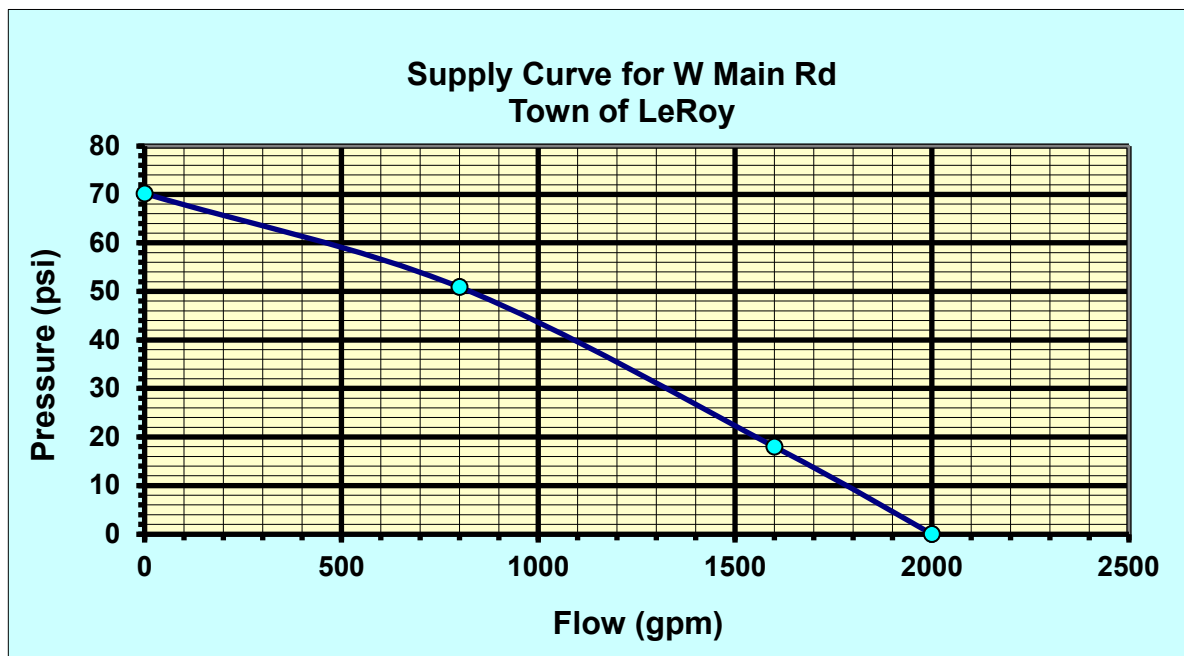
Main: 12" DI

Elevation: 888'

Pressure Zone: 1070

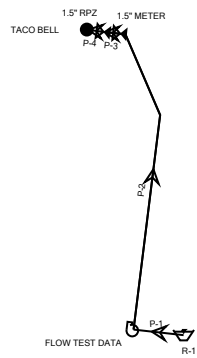
Supply Curve

<u>Q</u>	<u>PSI</u>	<u>H</u>
0	70.2	162.2
800	50.9	117.6
1600	18	41.6
2000	0	0



The pressure and flow data provided herein represents a computer simulated average estimated value at this location in the distribution system from known and/or estimated system variables. The flow values indicated by the supply curve may vary greatly depending on demands, operational parameters, system configurations, subsequent modifications and other related criteria.

DOMESTIC



Junction Table - Time: 0.00 hours

ID	Label	Elevation (ft)	Demand Collection	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
34	TACO BELL	885.50	<Collection: 1 item>	50	1,004.07	51

Pipe Table - Time: 0.00 hours

Pressure (Maximum at Start Node, Transient) (psi)	ID	Label	Start Node	Stop Node	Diameter (in)
(N/A)	33	P-1	R-1	FLOW TEST DATA	48.0
(N/A)	39	P-4	1.5" RPZ	TACO BELL	6.0
(N/A)	41	P-2	FLOW TEST DATA	1.5" METER	1.5
(N/A)	42	P-3	1.5" METER	1.5" RPZ	6.0
Material	Hazen-Williams C	Flow (gpm)	Headloss Gradient (ft/ft)		
Ductile Iron	150.0	50	0.000		
Ductile Iron	130.0	50	0.000		
HDPE	140.0	50	0.216		
Ductile Iron	130.0	50	0.000		

Pump Table - Time: 0.00 hours

ID	Label	Elevation (ft)	Pump Definition	Hydraulic Grade (Suction) (ft)	Hydraulic Grade (Discharge) (ft)
32	FLOW TEST DATA	883.74	Hydrant Flow Test	883.74	1,045.21
Flow (Total) (gpm)	Pump Head (ft)				
50	161.47				

Reservoir Table - Time: 0.00 hours

ID	Label	Elevation (ft)	Zone	Flow (Out net) (gpm)	Hydraulic Grade (ft)
31	R-1	883.74	<None>	50	883.74

GPV Table - Time: 0.00 hours

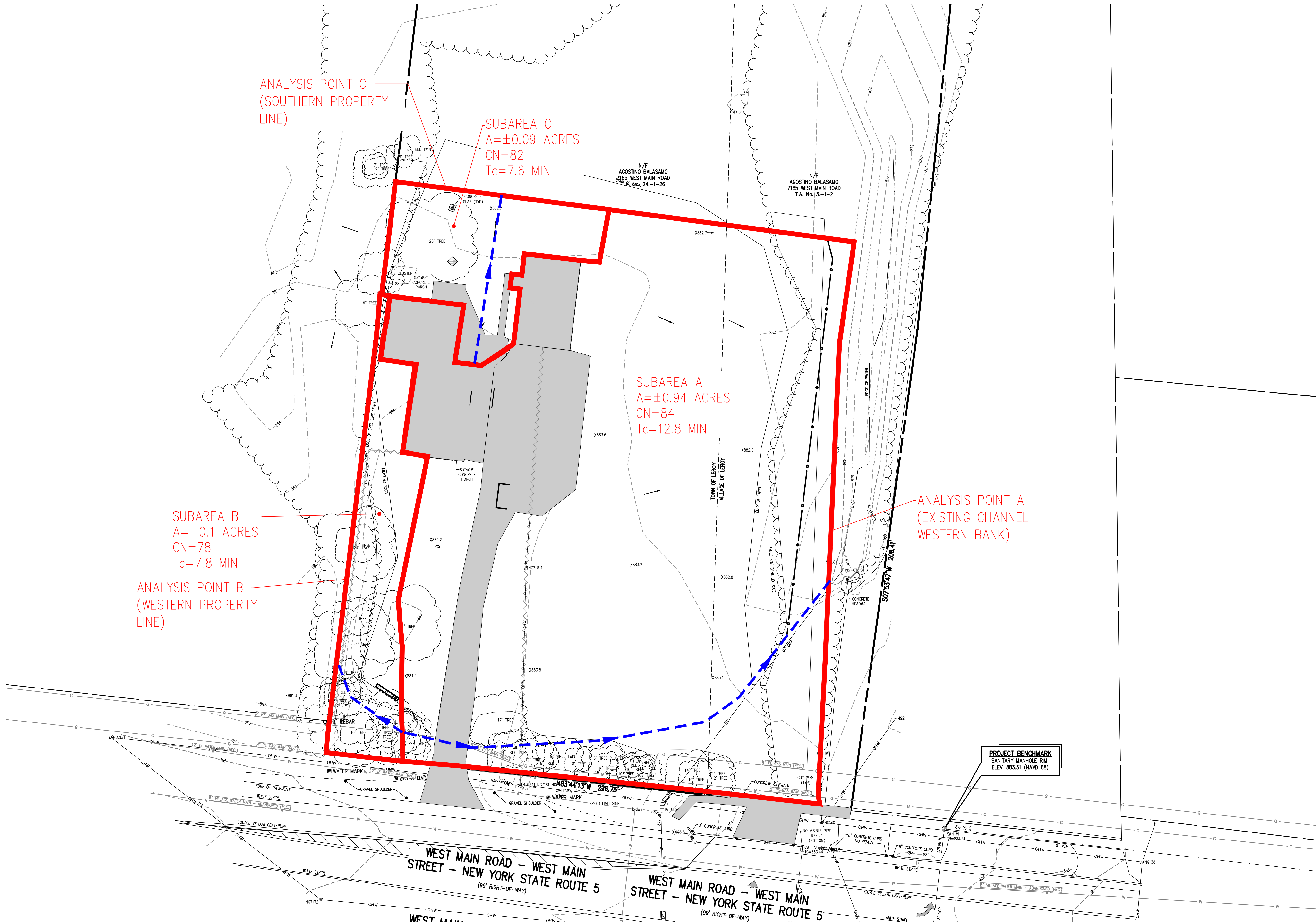
ID	Label	Elevation (ft)	Diameter (Valve) (in)	Minor Loss Coefficient (Local)	General Purpose Valve Headloss Curve	Flow (gpm)
37	1.5" RPZ	885.50	1.5	0.000	1.5" WATTS RPZ	50
40	1.5" METER	885.50	1.5	0.000	1.5" Neptune T-10 Meter	50

GPV Table - Time: 0.00 hours

Hydraulic Grade (From) (ft)	Hydraulic Grade (To) (ft)	Headloss (ft)
1,010.42	1,004.07	6.35
1,016.43	1,010.43	6.01

Appendix B

Stormwater Management



Drawing Alteration
The following is an excerpt from the New York State Education Law Article 145 Section 7209 and applies to this drawing:
"It is a violation of this law for any person, unless he is acting under the direction of a licensed professional engineer or land surveyor to alter any item in any way. If an item bearing the seal of an engineer or land surveyor is altered, the altering engineer or land surveyor shall affix to the item his seal and the notation "altered by" followed by his signature and the date of such alteration, and a specific description of the alteration."

7	6	5	4	3	2	1	REVISIONS	DATE	BY

BME ASSOCIATES
ENGINEERS • SURVEYORS • LANDSCAPE ARCHITECTS
800 PARKER HILL DRIVE, SUITE 300
ROCHESTER, NEW YORK 14625
WWW.BMEPC.COM
565-377-7360

LERoy TACO BELL
PROJECT LOCATION CLIENT
TOWN/VILLAGE OF LEROY, GENESSEE COUNTY, NEW YORK STATE
INDUS. HOSPITALITY GROUP
NEW PARKER TRACT
ROCHESTER, NEW YORK, 14625

DRAWING TITLE
EXISTING CONDITIONS

PROJECT MANAGER
P. VARS
PROJECT ENGINEER
R. SPURR
DRAWN BY
R. SPURR
SCALE
1" = 20'
DATE ISSUED
AUGUST 6, 2026
PROJECT NO.

3036
DRAWING NO.
FIGURE 03

Drawing Alteration
The following is an excerpt from the New York State Education Law Article 145 Section 7209 and applies to this drawing:
"It is a violation of this law for any person, unless he is acting under the direction of a licensed professional engineer or land surveyor to alter any item in any way. If an item bearing the seal of an engineer or land surveyor is altered, the altering engineer or land surveyor shall affix to the item his seal and the notation "altered by" followed by his signature and the date of such alteration, and a specific description of the alteration."

REVISIONS		DATE	BY
7			
6			
5			
4			
3			
2			
1			

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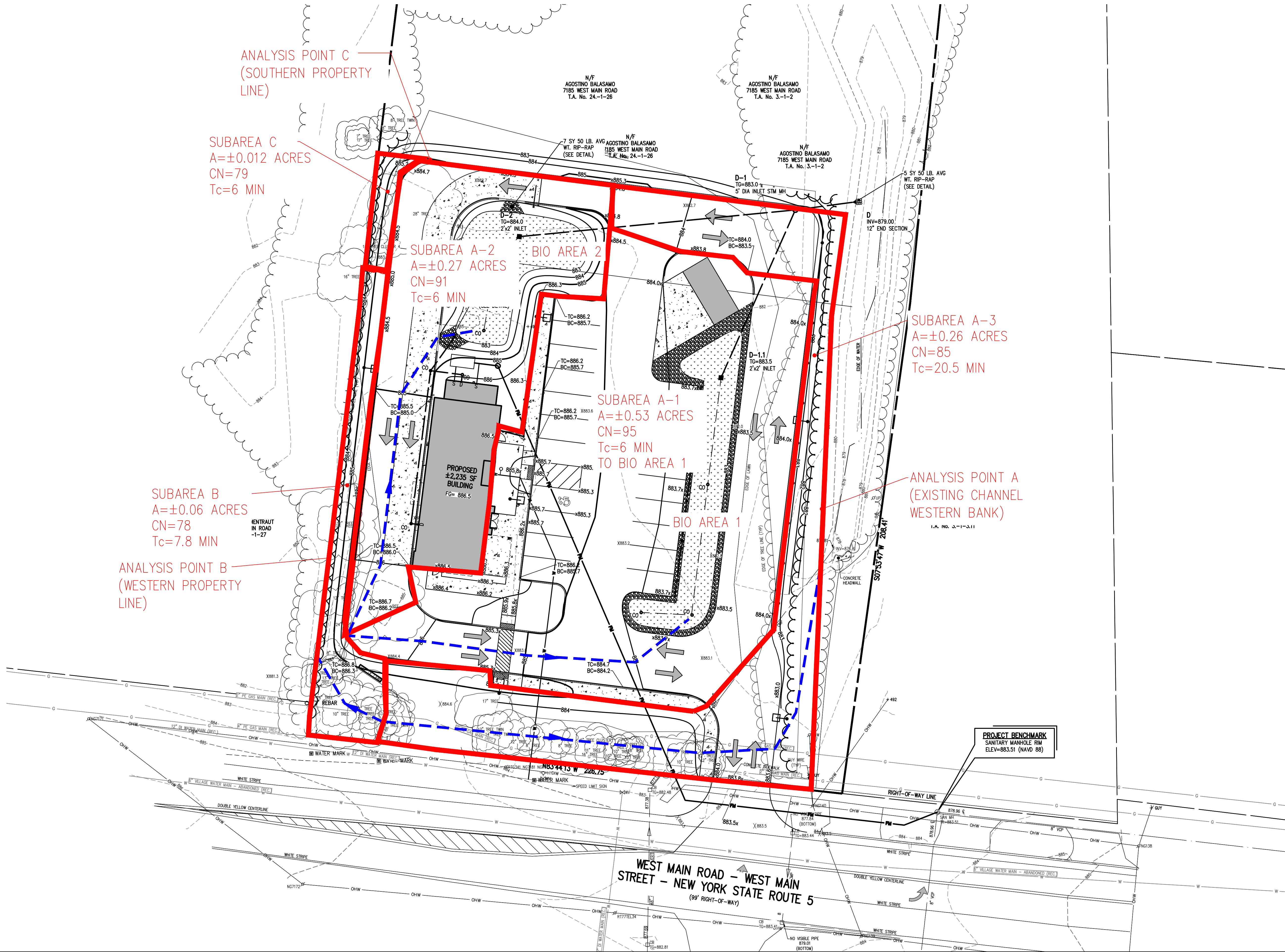
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TOWN/VILLAGE OF LEROY, GENESSEE COUNTY, NEW YORK STATE
INDUS. WASTE TREATMENT PLANT
NEW PARKER TRACT
ROCHESTER, NEW YORK, 14625

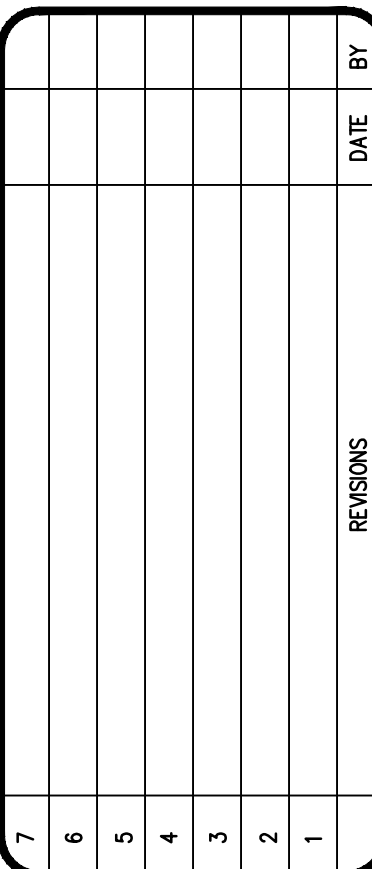
LOCATION
RANGE 1, TOWN LOT 3 CRANE TRACT, TAX MAP NUMBER 24-1-26 & 3-1-2

CLIENT
PROJECT MANAGER
P. VARS
PROJECT ENGINEER
R. SPURR
DRAWN BY
X
SCALE
1" = 20'
DATE ISSUED
AUGUST 6, 2026
PROJECT NO.

DRAWING NO.
3036
FIGURE 04

PROPOSED CONDITIONS





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800 PARKER HILL DRIVE, SUITE 300
ROCHESTER, NEW YORK 14625

WWW.BMEPC.COM
585-377-7360

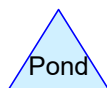
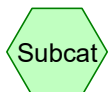
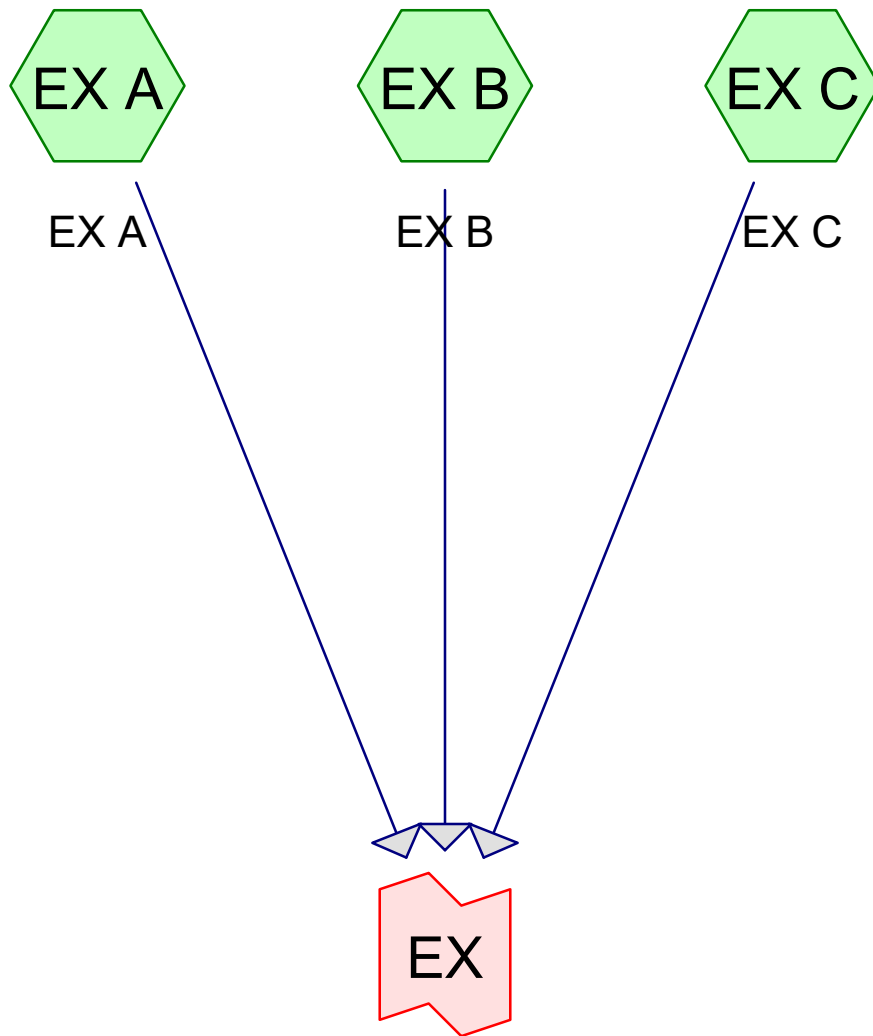
LERROY TACO BELL
TACO BELL
10000 LERROY RD.
LERROY, NEW YORK 14625

PROJECT	LOCATION	CLIENT	DRAWING TITLE
---------	----------	--------	---------------

PROJECT MANAGER	
P. VARS	
PROJECT ENGINEER	
R. SPURR	
DRAWN BY	
X	
SCALE	DATE ISSUED
1" = 20'	AUGUST 6, 2026
PROJECT NO.	

3036
DRAWING NO.
FIGURE 05

EXISTING CONDITIONS



Routing Diagram for 3036 Drainage

Prepared by BME Associates, Printed 9/22/2026

HydroCAD® 10.20-7a s/n 09860 © 2025 HydroCAD Software Solutions LLC

3036 Drainage

Prepared by BME Associates

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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Printed 9/22/2026

Page 2

Summary for Subcatchment EX A: EX A

Runoff = 0.63 cfs @ 12.14 hrs, Volume= 0.045 af, Depth> 0.57"
Routed to Link EX : COMBINED EX

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
* 0.220	98	impervious area
0.110	77	Woods, Good, HSG D
0.610	80	>75% Grass cover, Good, HSG D
0.940	84	Weighted Average
0.720		76.60% Pervious Area
0.220		23.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	24	0.0200	0.10		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.3	12	0.0150	0.66		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 2.14"
8.0	60	0.0200	0.13		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.4	70	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
0.3	20	0.0500	1.12		Shallow Concentrated Flow, Shallow Conc Woodland Kv= 5.0 fps
12.8	186	Total			

3036 Drainage

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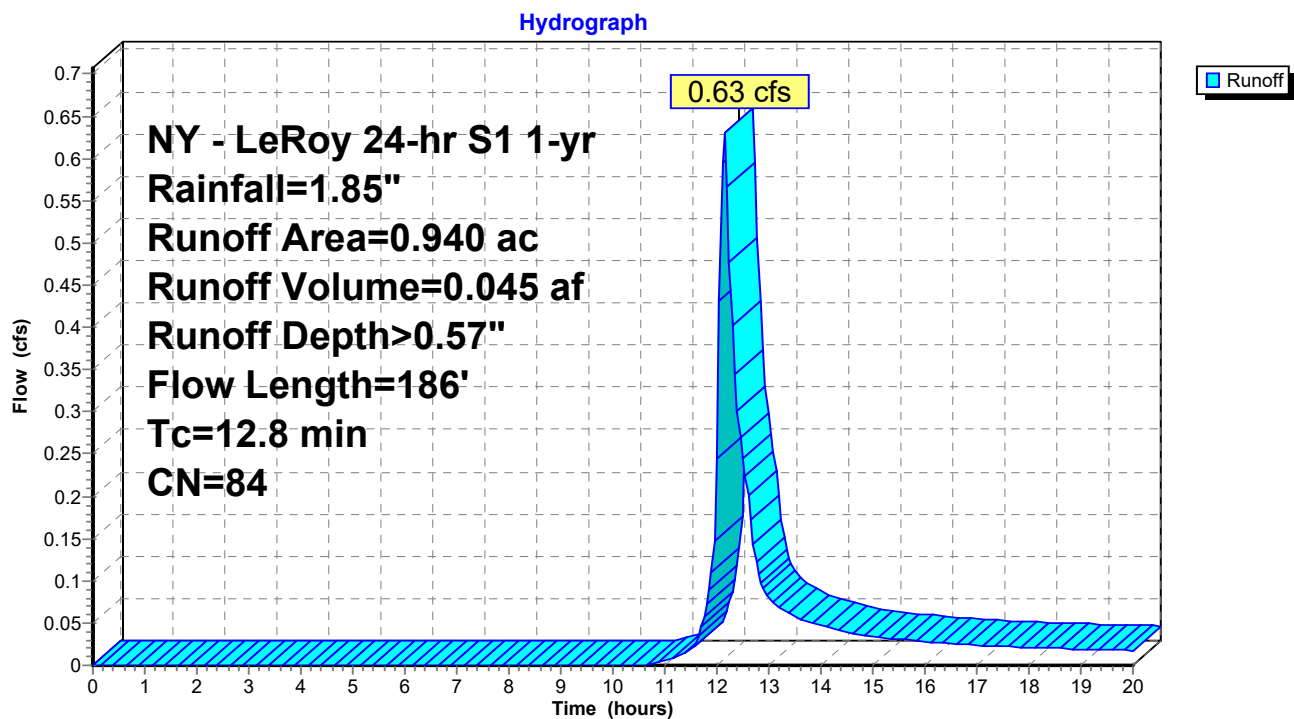
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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 3

Subcatchment EX A: EX A



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 4

Summary for Subcatchment EX B: EX B

Runoff = 0.04 cfs @ 12.08 hrs, Volume= 0.003 af, Depth> 0.35"
Routed to Link EX : COMBINED EX

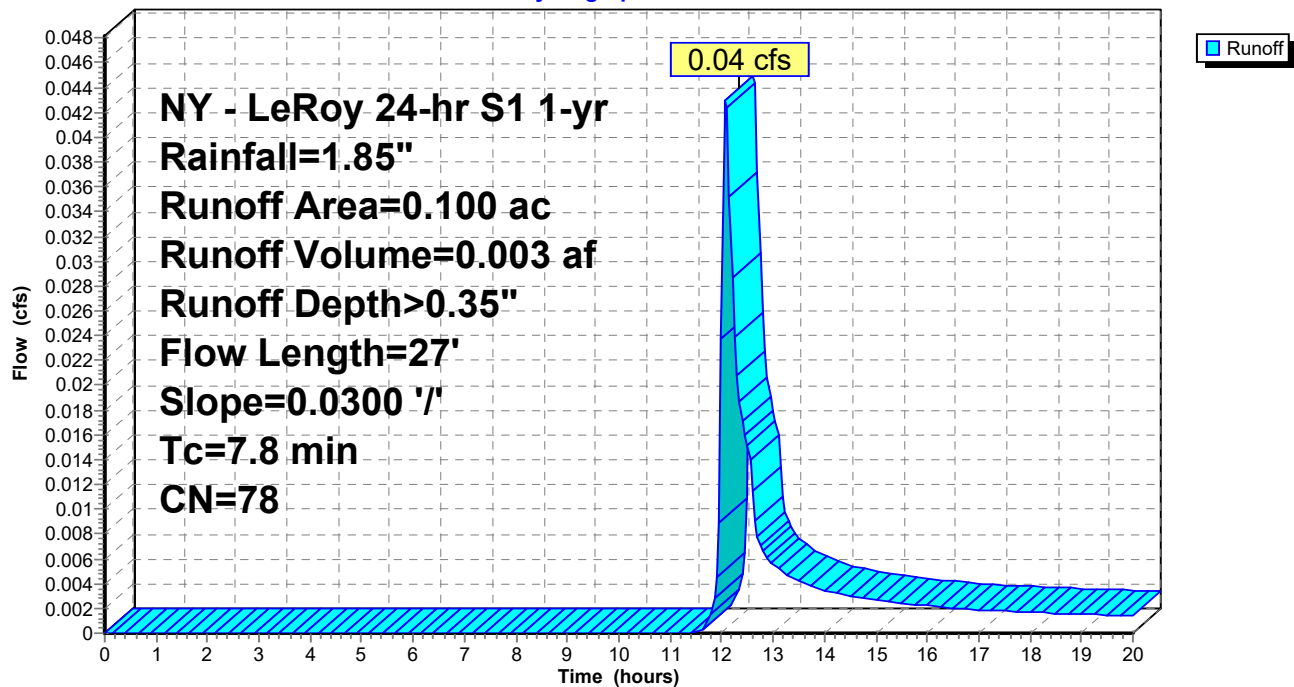
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
0.045	80	>75% Grass cover, Good, HSG D
0.055	77	Woods, Good, HSG D
0.100	78	Weighted Average
0.100		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	27	0.0300	0.06		Sheet Flow, Sheet FLOW Woods: Light underbrush n= 0.400 P2= 2.14"

Subcatchment EX B: EX B

Hydrograph



3036 Drainage

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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 5

Summary for Subcatchment EX C: EX C

Runoff = 0.05 cfs @ 12.18 hrs, Volume= 0.004 af, Depth> 0.49"
Routed to Link EX : COMBINED EX

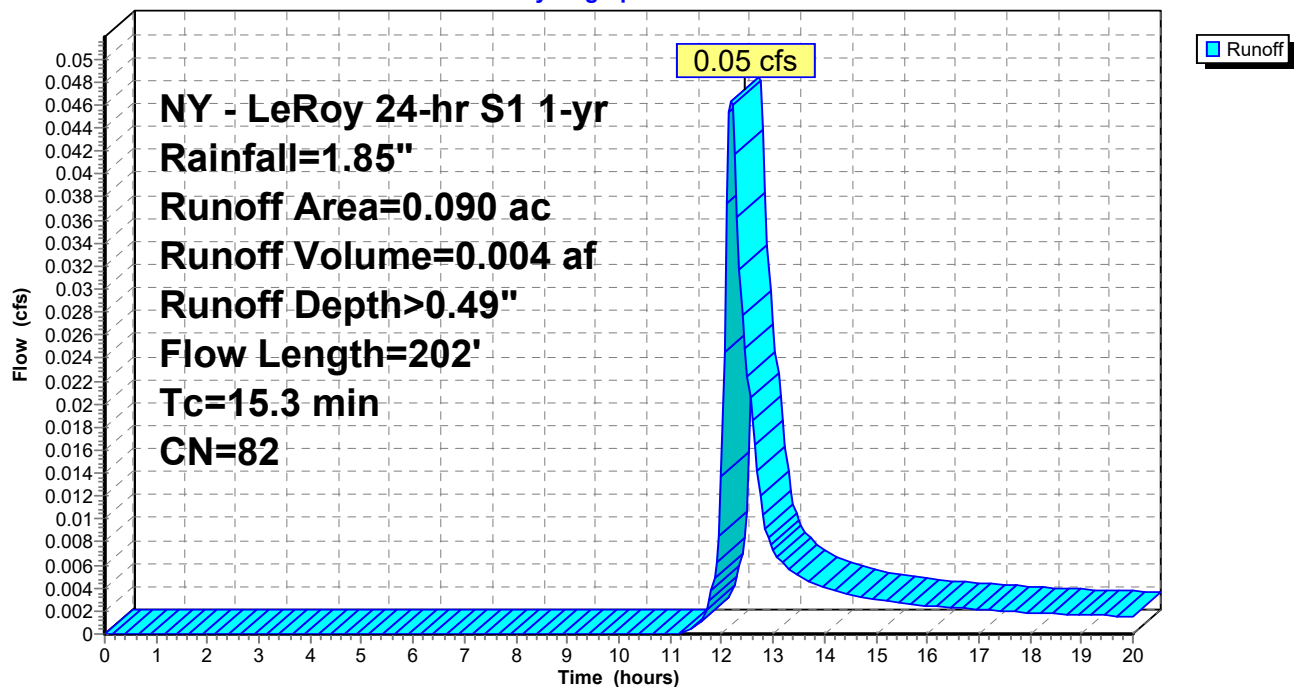
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
* 0.010	98	impervious
0.080	80	>75% Grass cover, Good, HSG D
0.090	82	Weighted Average
0.080		88.89% Pervious Area
0.010		11.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.5	12	0.0150	0.08		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.3	12	0.0150	0.66		Sheet Flow, Sheet Flow Impv Smooth surfaces n= 0.011 P2= 2.14"
12.0	100	0.0200	0.14		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.5	78	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
15.3	202	Total			

Subcatchment EX C: EX C

Hydrograph



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 6

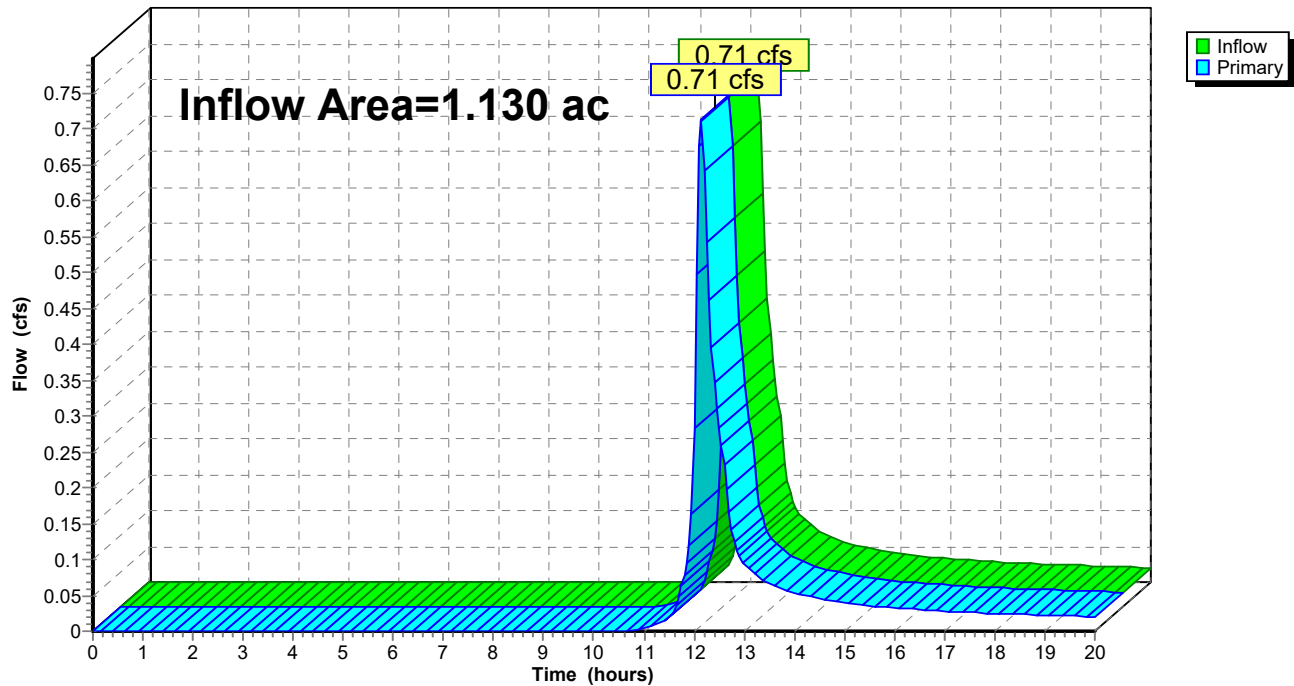
Summary for Link EX: COMBINED EX

Inflow Area = 1.130 ac, 20.35% Impervious, Inflow Depth > 0.55" for 1-yr event
Inflow = 0.71 cfs @ 12.14 hrs, Volume= 0.052 af
Primary = 0.71 cfs @ 12.14 hrs, Volume= 0.052 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs

Link EX: COMBINED EX

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 7

Summary for Subcatchment EX A: EX A

Runoff = 1.57 cfs @ 12.14 hrs, Volume= 0.110 af, Depth> 1.41"
 Routed to Link EX : COMBINED EX

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
 NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
* 0.220	98	impervious area
0.110	77	Woods, Good, HSG D
0.610	80	>75% Grass cover, Good, HSG D
0.940	84	Weighted Average
0.720		76.60% Pervious Area
0.220		23.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	24	0.0200	0.10		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.3	12	0.0150	0.66		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 2.14"
8.0	60	0.0200	0.13		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.4	70	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
0.3	20	0.0500	1.12		Shallow Concentrated Flow, Shallow Conc Woodland Kv= 5.0 fps
12.8	186	Total			

3036 Drainage

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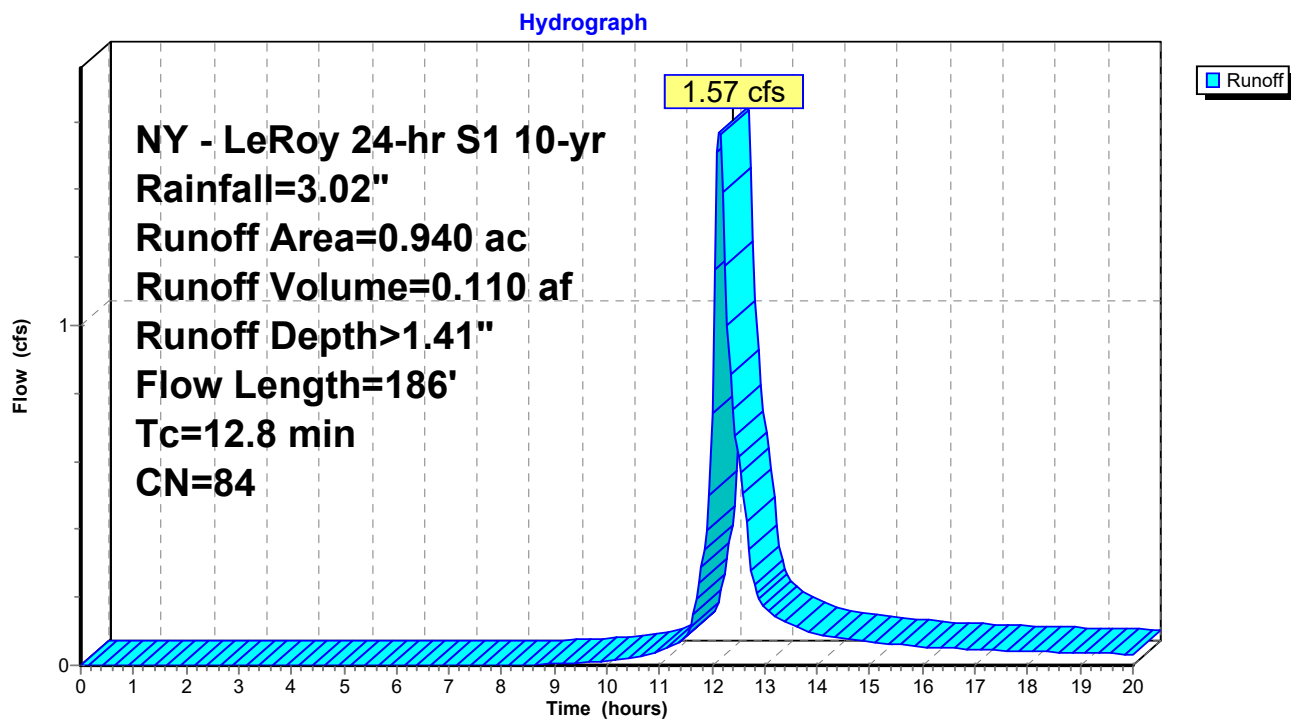
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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 8

Subcatchment EX A: EX A



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 9

Summary for Subcatchment EX B: EX B

Runoff = 0.15 cfs @ 12.07 hrs, Volume= 0.009 af, Depth> 1.04"
Routed to Link EX : COMBINED EX

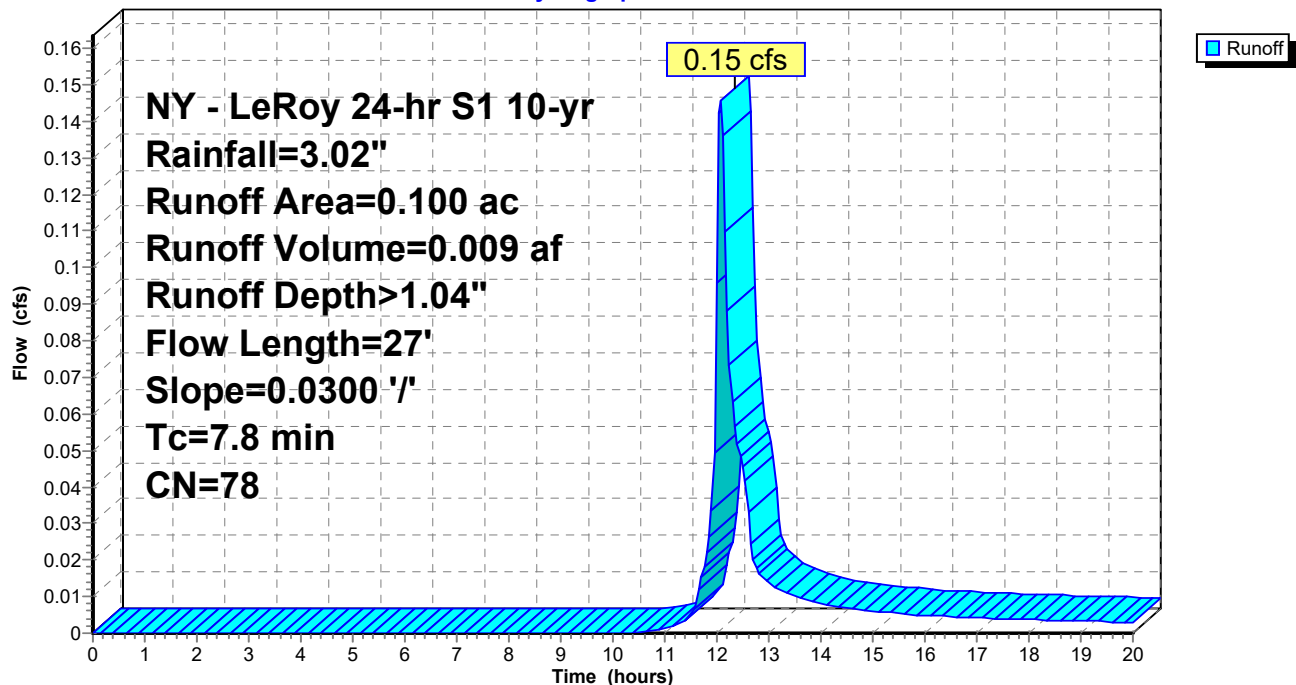
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
0.045	80	>75% Grass cover, Good, HSG D
0.055	77	Woods, Good, HSG D
0.100	78	Weighted Average
0.100		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	27	0.0300	0.06		Sheet Flow, Sheet FLOW
Woods: Light underbrush n= 0.400 P2= 2.14"					

Subcatchment EX B: EX B

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 10

Summary for Subcatchment EX C: EX C

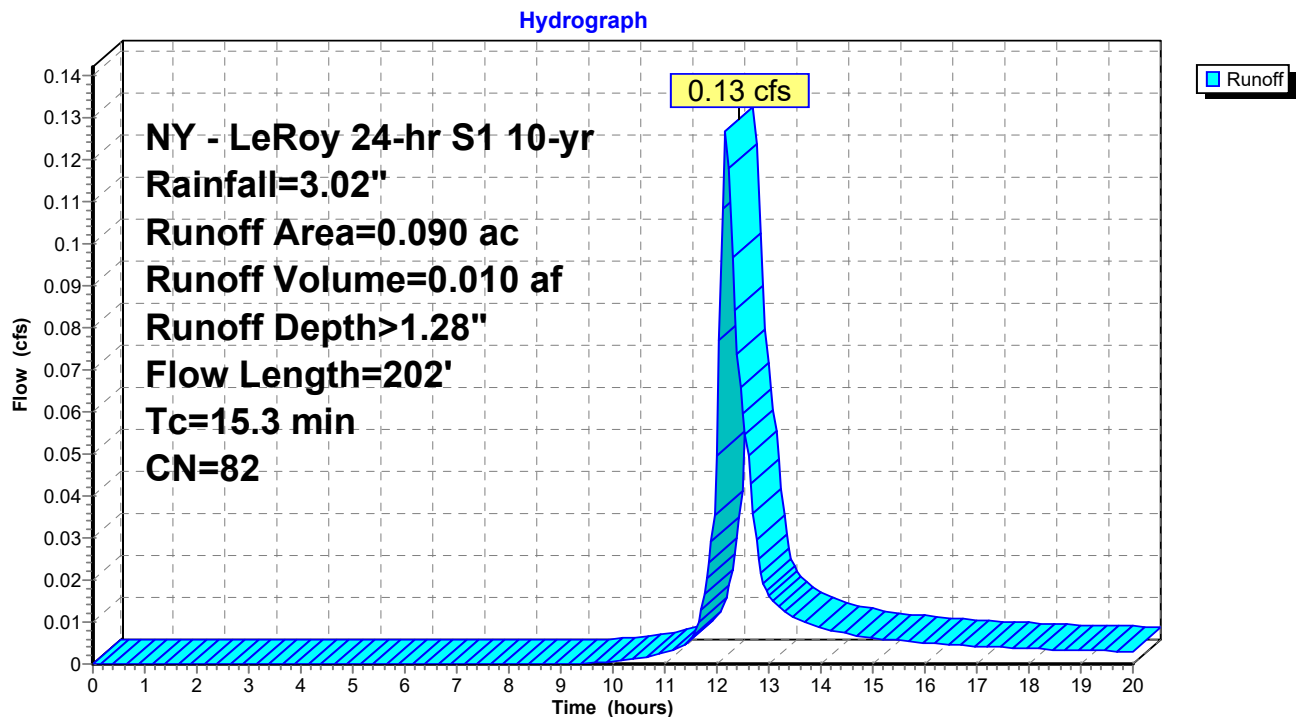
Runoff = 0.13 cfs @ 12.17 hrs, Volume= 0.010 af, Depth> 1.28"
Routed to Link EX : COMBINED EX

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
* 0.010	98	impervious
0.080	80	>75% Grass cover, Good, HSG D
0.090	82	Weighted Average
0.080		88.89% Pervious Area
0.010		11.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.5	12	0.0150	0.08		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.3	12	0.0150	0.66		Sheet Flow, Sheet Flow Impv Smooth surfaces n= 0.011 P2= 2.14"
12.0	100	0.0200	0.14		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.5	78	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
15.3	202	Total			

Subcatchment EX C: EX C



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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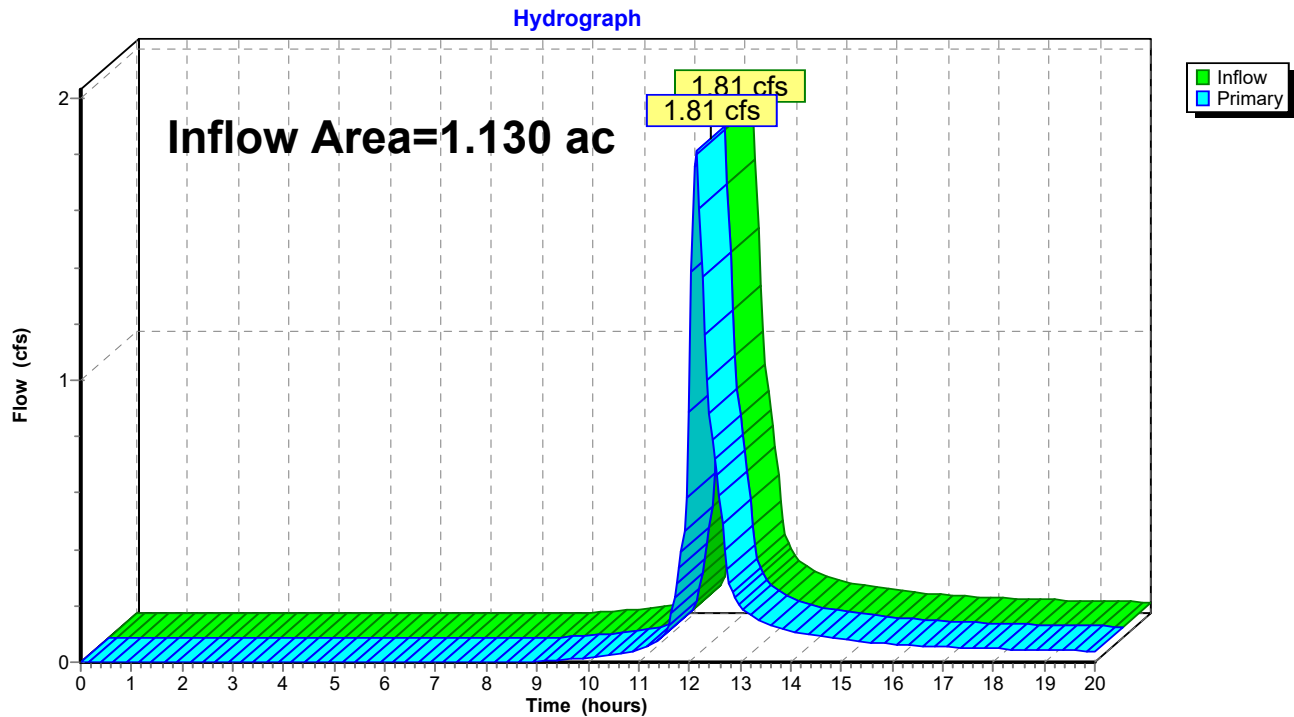
Page 11

Summary for Link EX: COMBINED EX

Inflow Area = 1.130 ac, 20.35% Impervious, Inflow Depth > 1.37" for 10-yr event
Inflow = 1.81 cfs @ 12.13 hrs, Volume= 0.129 af
Primary = 1.81 cfs @ 12.13 hrs, Volume= 0.129 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs

Link EX: COMBINED EX



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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 12

Summary for Subcatchment EX A: EX A

Runoff = 3.33 cfs @ 12.13 hrs, Volume= 0.234 af, Depth> 2.99"
 Routed to Link EX : COMBINED EX

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
 NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
* 0.220	98	impervious area
0.110	77	Woods, Good, HSG D
0.610	80	>75% Grass cover, Good, HSG D
0.940	84	Weighted Average
0.720		76.60% Pervious Area
0.220		23.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	24	0.0200	0.10		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.3	12	0.0150	0.66		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 2.14"
8.0	60	0.0200	0.13		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.4	70	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
0.3	20	0.0500	1.12		Shallow Concentrated Flow, Shallow Conc Woodland Kv= 5.0 fps
12.8	186	Total			

3036 Drainage

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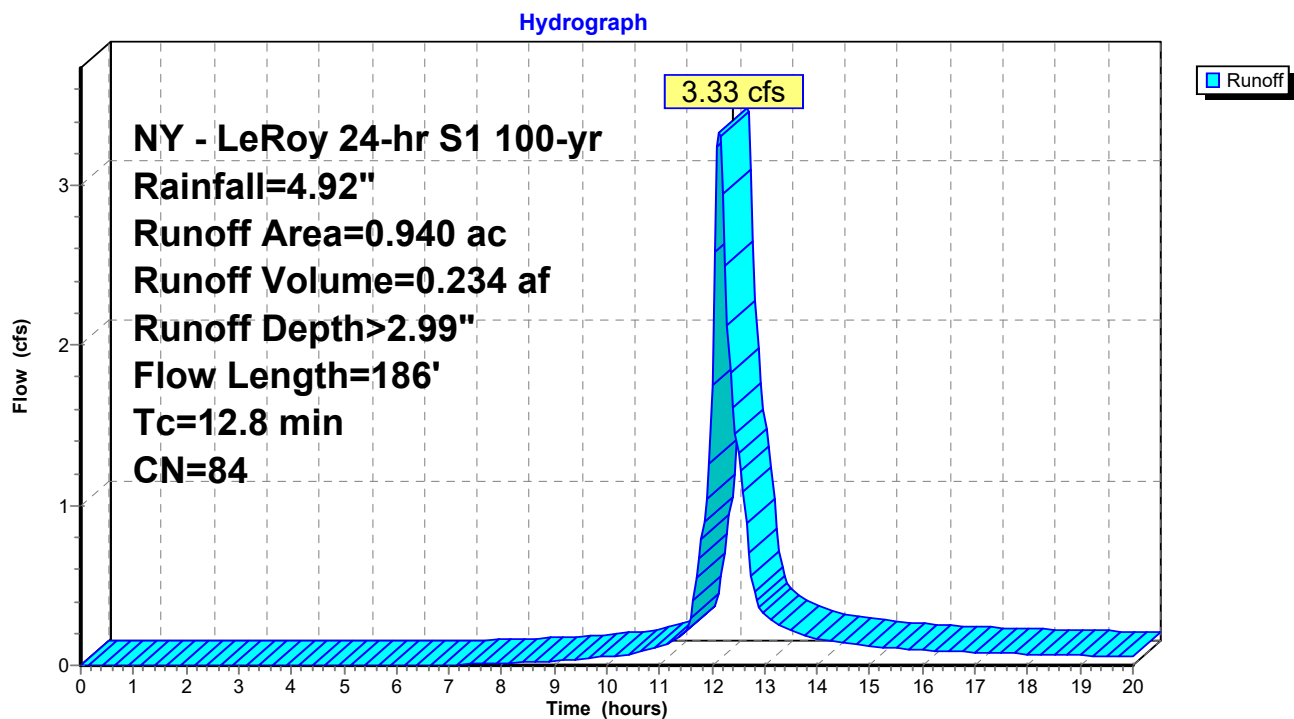
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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 13

Subcatchment EX A: EX A



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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 14

Summary for Subcatchment EX B: EX B

Runoff = 0.35 cfs @ 12.07 hrs, Volume= 0.020 af, Depth> 2.46"
Routed to Link EX : COMBINED EX

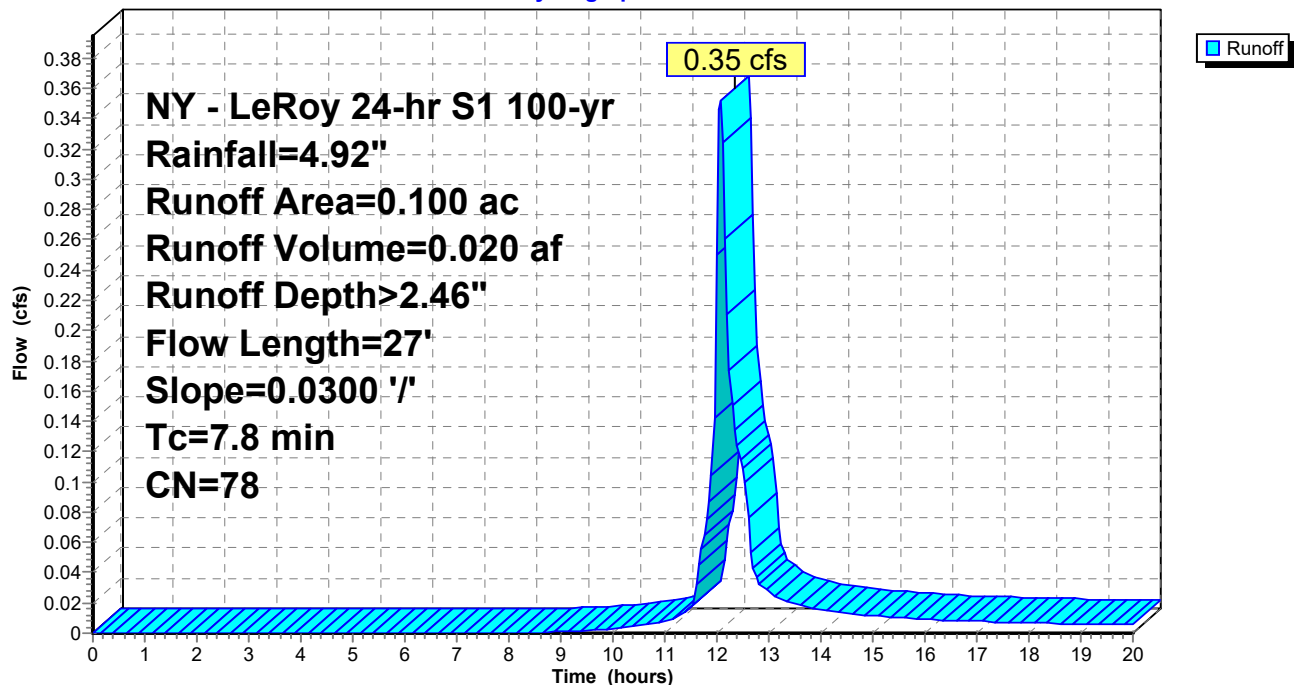
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
0.045	80	>75% Grass cover, Good, HSG D
0.055	77	Woods, Good, HSG D
0.100	78	Weighted Average
0.100		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	27	0.0300	0.06		Sheet Flow, Sheet FLOW
					Woods: Light underbrush n= 0.400 P2= 2.14"

Subcatchment EX B: EX B

Hydrograph



3036 Drainage

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 15

Summary for Subcatchment EX C: EX C

Runoff = 0.28 cfs @ 12.17 hrs, Volume= 0.021 af, Depth> 2.80"
Routed to Link EX : COMBINED EX

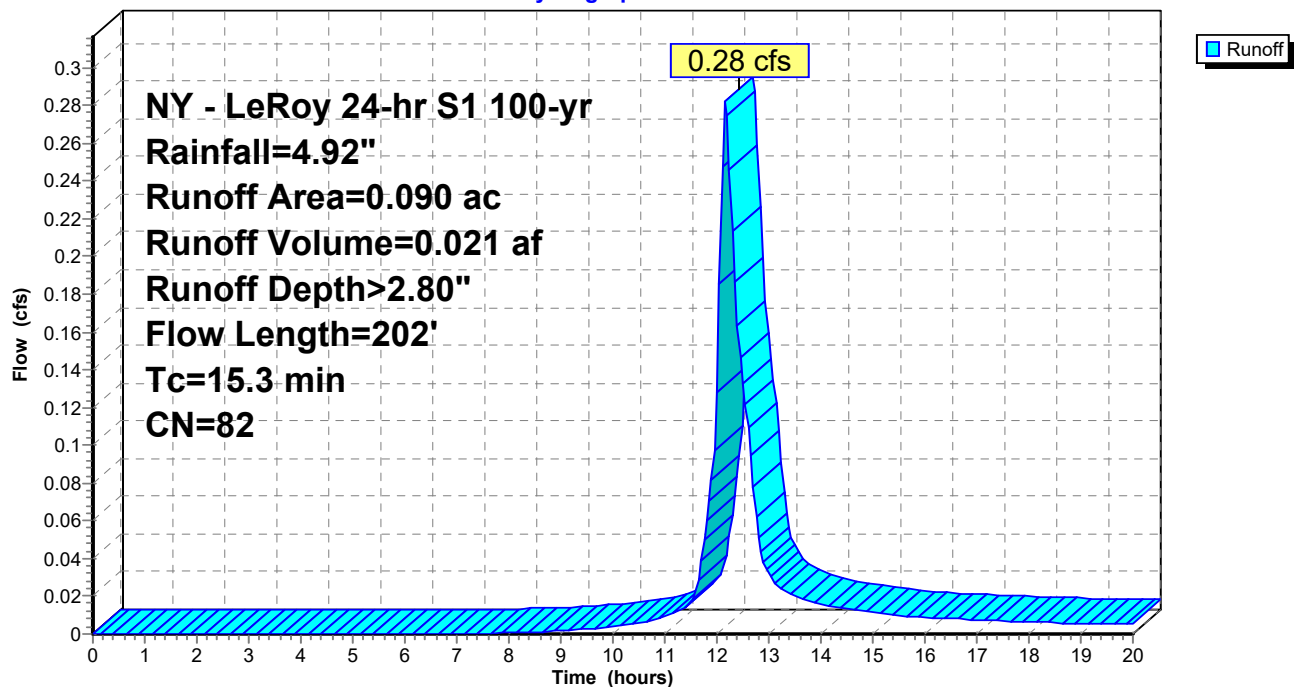
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
* 0.010	98	impervious
0.080	80	>75% Grass cover, Good, HSG D
0.090	82	Weighted Average
0.080		88.89% Pervious Area
0.010		11.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.5	12	0.0150	0.08		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.3	12	0.0150	0.66		Sheet Flow, Sheet Flow Impv Smooth surfaces n= 0.011 P2= 2.14"
12.0	100	0.0200	0.14		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.5	78	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
15.3	202	Total			

Subcatchment EX C: EX C

Hydrograph



3036 Drainage

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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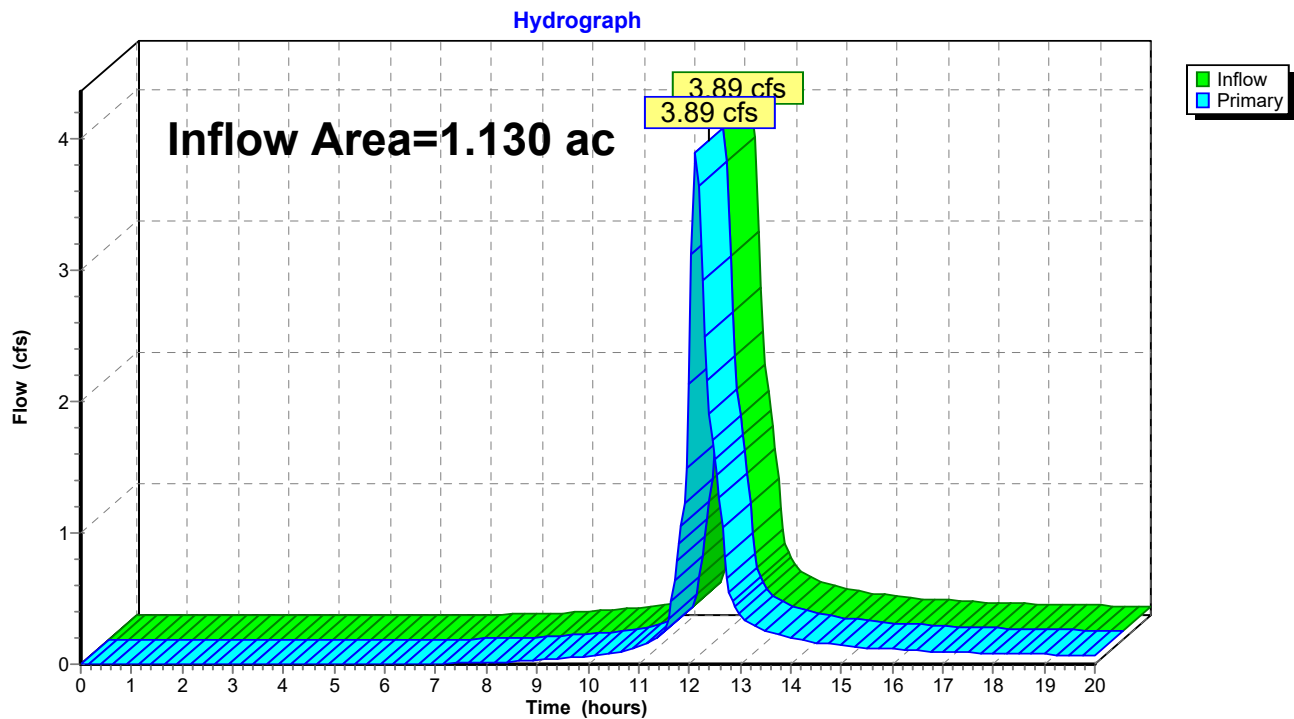
Page 16

Summary for Link EX: COMBINED EX

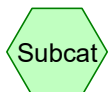
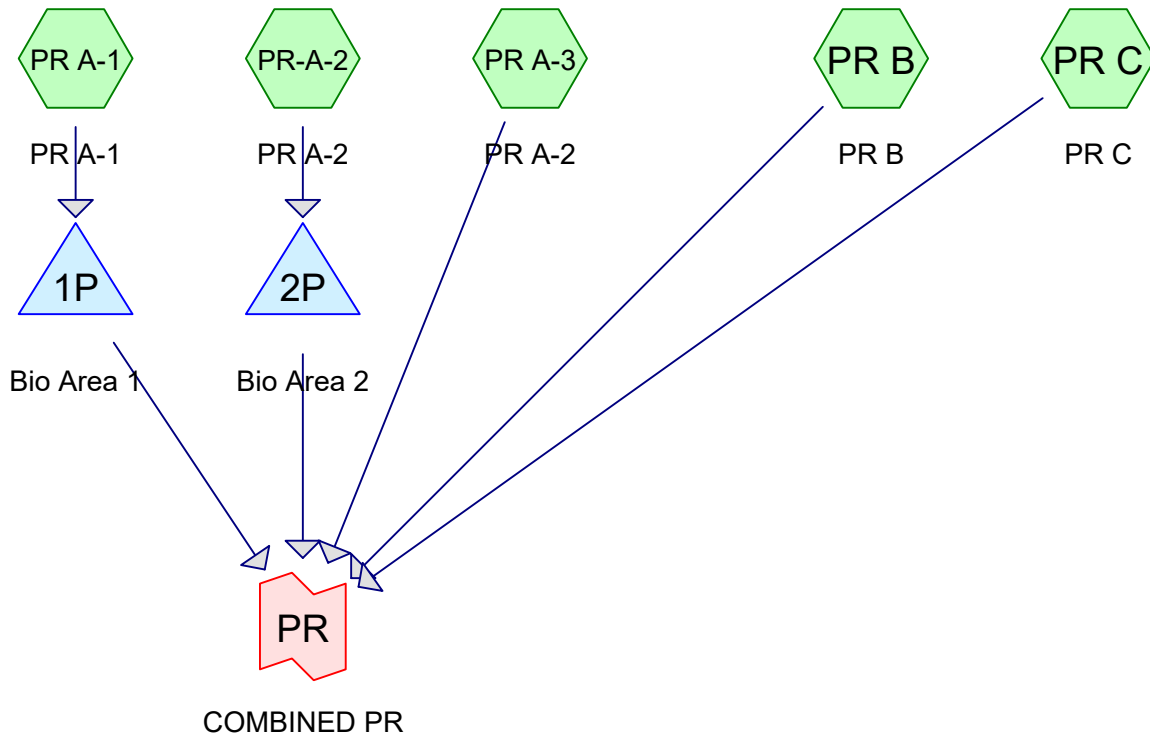
Inflow Area = 1.130 ac, 20.35% Impervious, Inflow Depth > 2.93" for 100-yr event
Inflow = 3.89 cfs @ 12.13 hrs, Volume= 0.276 af
Primary = 3.89 cfs @ 12.13 hrs, Volume= 0.276 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs

Link EX: COMBINED EX



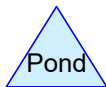
**PROPOSED
CONDITIONS**



Subcat



Reach



Pond



Link

Routing Diagram for 3036 Drainage

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3036 Drainage

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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 2

Summary for Subcatchment PR A-1: PR A-1

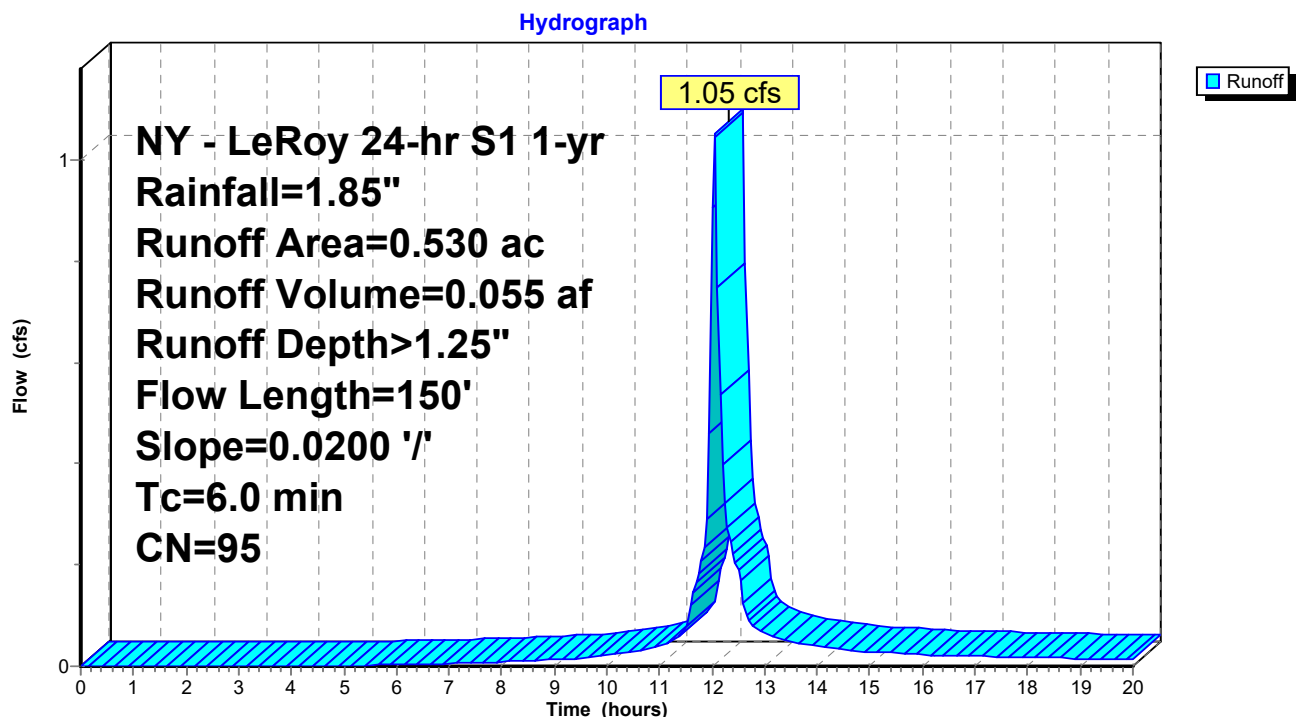
Runoff = 1.05 cfs @ 12.04 hrs, Volume= 0.055 af, Depth> 1.25"
Routed to Pond 1P : Bio Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
* 0.430	98	Impervious
0.100	80	>75% Grass cover, Good, HSG D
0.530	95	Weighted Average
0.100		18.87% Pervious Area
0.430		81.13% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.0	150	0.0200	1.22		Sheet Flow, Sheet Flow
					Smooth surfaces n= 0.011 P2= 2.14"
2.0	150	Total, Increased to minimum Tc = 6.0 min			

Subcatchment PR A-1: PR A-1



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 3

Summary for Subcatchment PR A-3: PR A-2

Runoff = 0.15 cfs @ 12.25 hrs, Volume= 0.013 af, Depth> 0.62"
Routed to Link PR : COMBINED PR

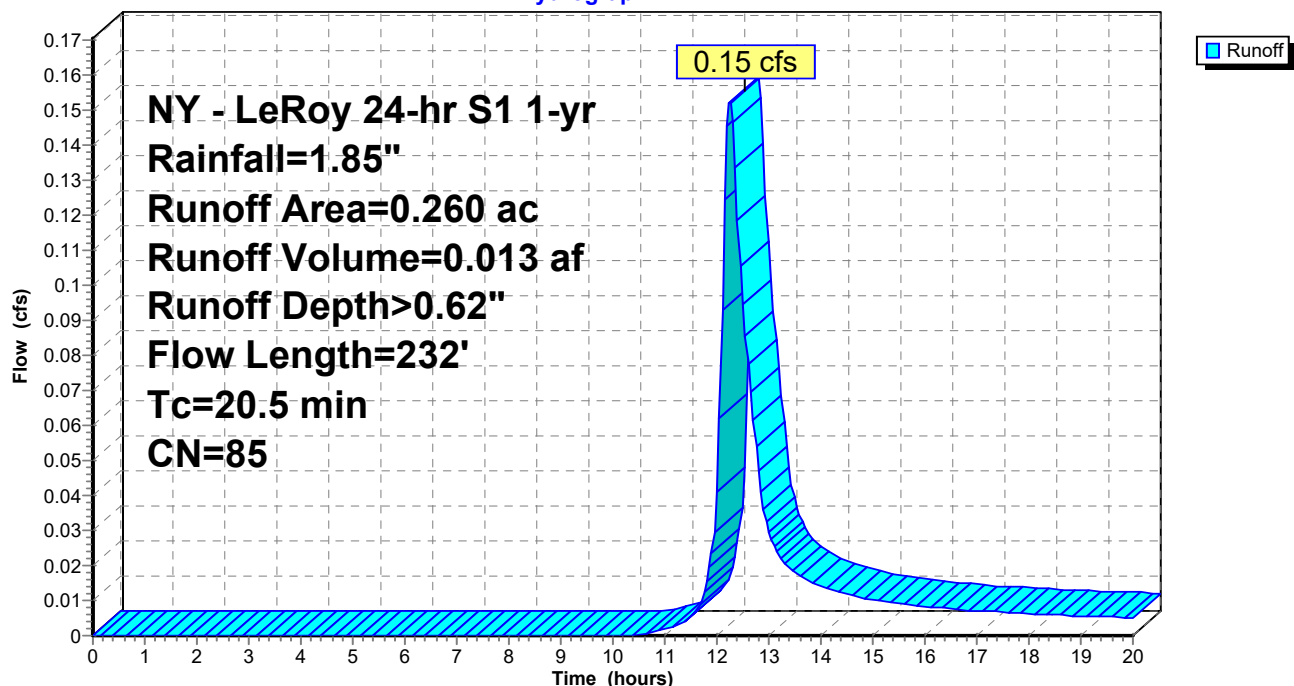
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
* 0.080	98	Impervious
0.090	77	Woods, Good, HSG D
0.090	80	>75% Grass cover, Good, HSG D
0.260	85	Weighted Average
0.180		69.23% Pervious Area
0.080		30.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.5	130	0.0100	0.11		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.5	24	0.0150	0.75		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 2.14"
0.5	78	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
20.5	232	Total			

Subcatchment PR A-3: PR A-2

Hydrograph



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 4

Summary for Subcatchment PR B: PR B

Runoff = 0.03 cfs @ 12.08 hrs, Volume= 0.002 af, Depth> 0.35"
Routed to Link PR : COMBINED PR

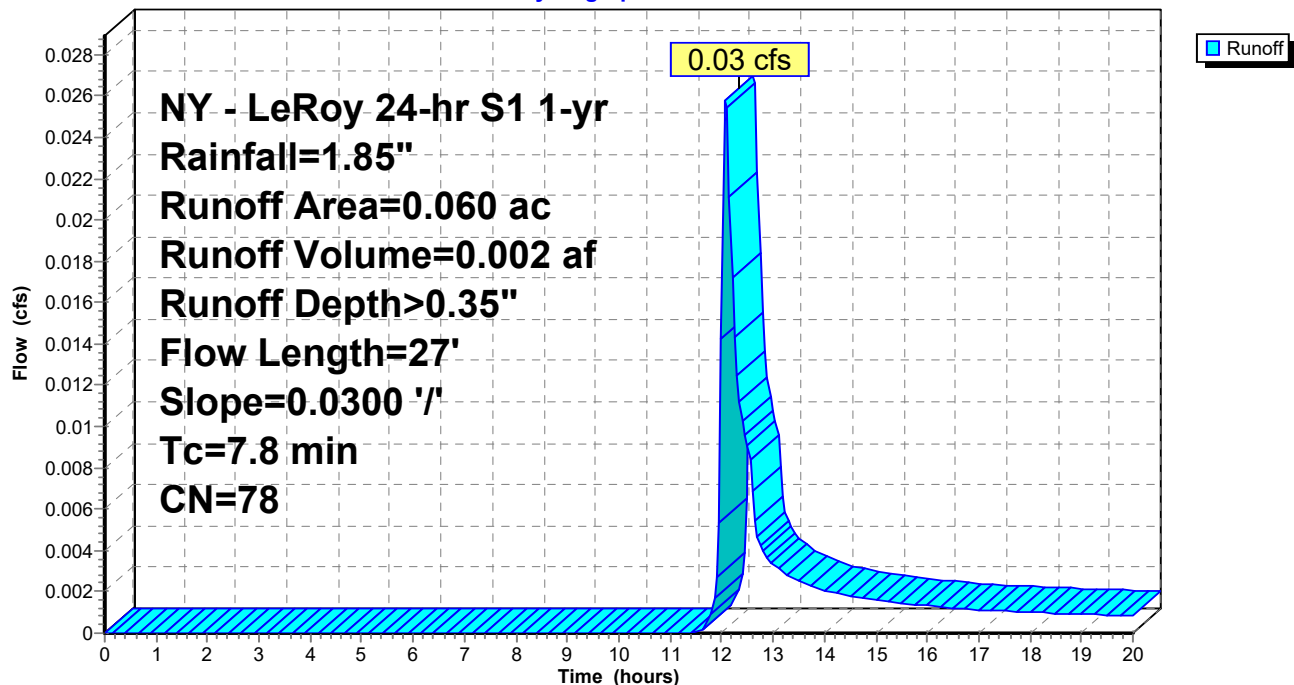
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
0.020	80	>75% Grass cover, Good, HSG D
0.040	77	Woods, Good, HSG D
0.060	78	Weighted Average
0.060		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	27	0.0300	0.06		Sheet Flow, Sheet Flow
					Woods: Light underbrush n= 0.400 P2= 2.14"

Subcatchment PR B: PR B

Hydrograph



3036 Drainage

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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 5

Summary for Subcatchment PR C: PR C

Runoff = 0.01 cfs @ 12.05 hrs, Volume= 0.000 af, Depth> 0.39"
Routed to Link PR : COMBINED PR

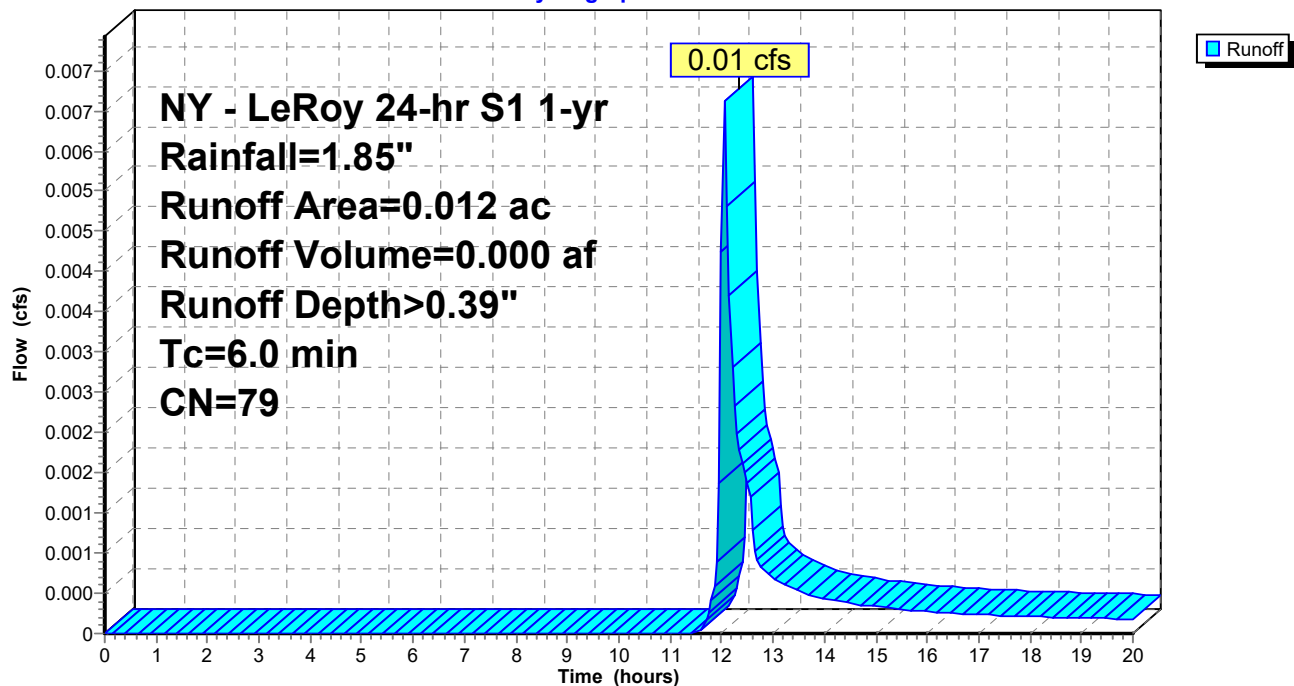
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
0.006	80	>75% Grass cover, Good, HSG D
0.006	77	Woods, Good, HSG D
0.012	79	Weighted Average
0.012		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR C: PR C

Hydrograph



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 6

Summary for Subcatchment PR-A-2: PR A-2

Runoff = 0.42 cfs @ 12.04 hrs, Volume= 0.021 af, Depth> 0.95"
Routed to Pond 2P : Bio Area 2

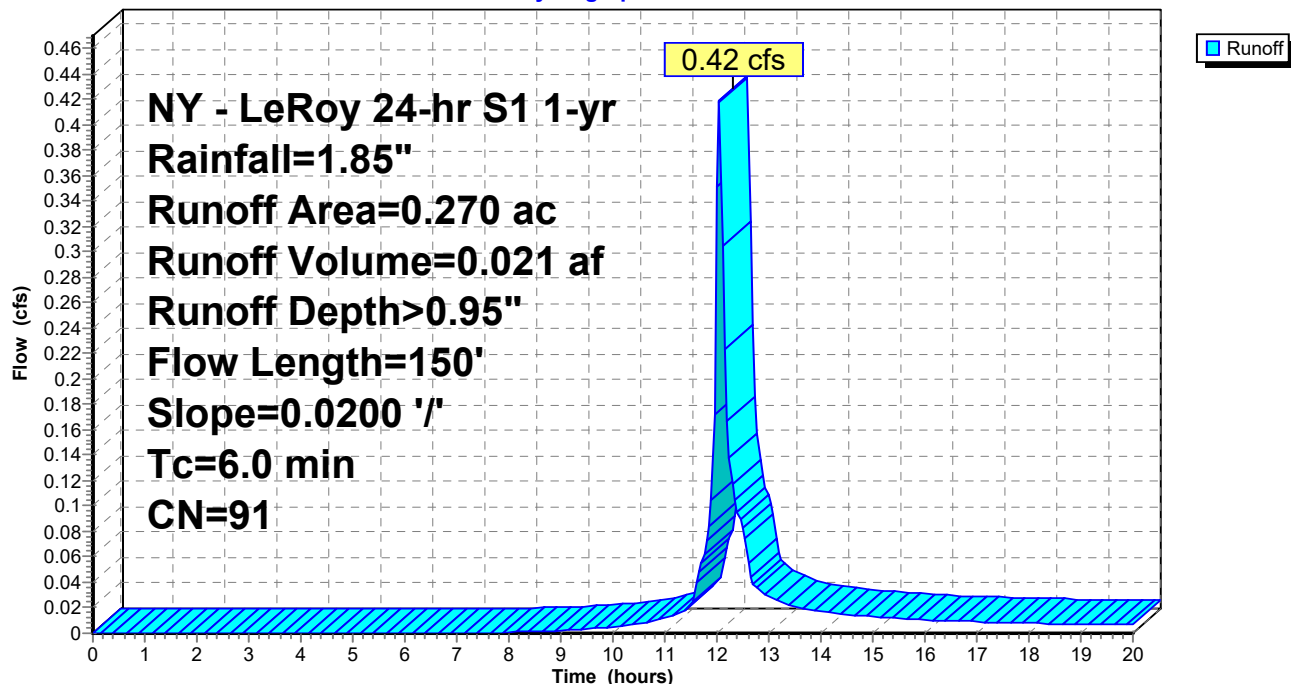
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

Area (ac)	CN	Description
* 0.170	98	Impervious
0.100	80	>75% Grass cover, Good, HSG D
0.270	91	Weighted Average
0.100		37.04% Pervious Area
0.170		62.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.0	150	0.0200	1.22		Sheet Flow, Sheet Flow
					Smooth surfaces n= 0.011 P2= 2.14"
2.0	150	Total, Increased to minimum Tc = 6.0 min			

Subcatchment PR-A-2: PR A-2

Hydrograph



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 7

Summary for Pond 1P: Bio Area 1

Inflow Area = 0.530 ac, 81.13% Impervious, Inflow Depth > 1.25" for 1-yr event
Inflow = 1.05 cfs @ 12.04 hrs, Volume= 0.055 af
Outflow = 0.05 cfs @ 13.45 hrs, Volume= 0.024 af, Atten= 95%, Lag= 84.8 min
Primary = 0.05 cfs @ 13.45 hrs, Volume= 0.024 af
Routed to Link PR : COMBINED PR

Routing by Stor-Ind method, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 882.56' @ 13.45 hrs Surf.Area= 2,064 sf Storage= 1,470 cf

Plug-Flow detention time= 195.3 min calculated for 0.024 af (44% of inflow)
Center-of-Mass det. time= 105.1 min (870.6 - 765.5)

Volume	Invert	Avail.Storage	Storage Description	
#1	879.50'	2,828 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
879.50	2,064	0.0	0	0
880.00	2,064	40.0	413	413
883.00	2,064	20.0	1,238	1,651
883.10	2,064	100.0	206	1,858
883.50	2,790	100.0	971	2,828

Device	Routing	Invert	Outlet Devices
#1	Primary	879.50'	8.0" Round Culvert L= 78.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 879.50' / 879.00' S= 0.0064 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	879.50'	0.500 in/hr BIO MEDIA over Surface area
#3	Device 1	882.50'	6.0" W x 6.0" H Vert. WEIR C= 0.600 Limited to weir flow at low heads
#4	Device 1	883.00'	24.0" x 24.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.05 cfs @ 13.45 hrs HW=882.56' (Free Discharge)

- 1=Culvert (Passes 0.05 cfs of 2.00 cfs potential flow)
- 2=BIO MEDIA (Exfiltration Controls 0.02 cfs)
- 3=WEIR (Orifice Controls 0.02 cfs @ 0.79 fps)
- 4=Grate (Controls 0.00 cfs)

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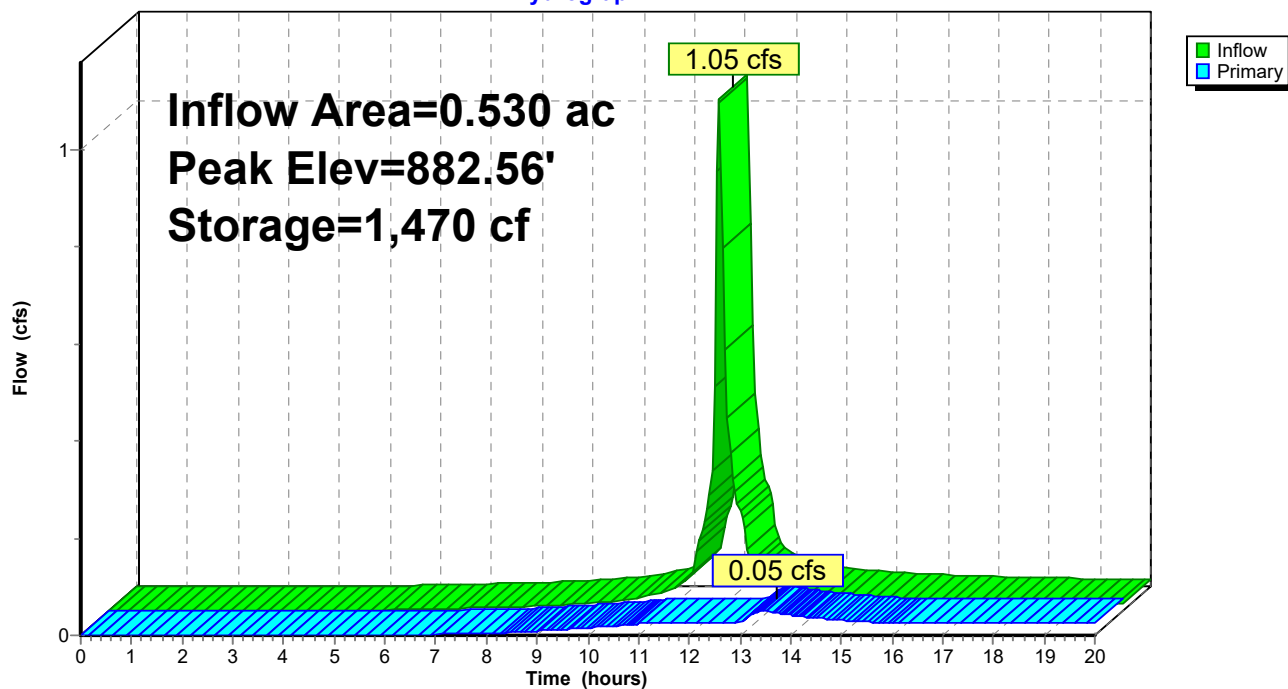
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 8

Pond 1P: Bio Area 1

Hydrograph



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 9

Summary for Pond 2P: Bio Area 2

Inflow Area = 0.270 ac, 62.96% Impervious, Inflow Depth > 0.95" for 1-yr event
Inflow = 0.42 cfs @ 12.04 hrs, Volume= 0.021 af
Outflow = 0.02 cfs @ 11.90 hrs, Volume= 0.015 af, Atten= 95%, Lag= 0.0 min
Primary = 0.02 cfs @ 11.90 hrs, Volume= 0.015 af
Routed to Link PR : COMBINED PR

Routing by Stor-Ind method, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 880.64' @ 13.55 hrs Surf.Area= 1,800 sf Storage= 517 cf

Plug-Flow detention time= 216.0 min calculated for 0.015 af (71% of inflow)
Center-of-Mass det. time= 148.3 min (935.8 - 787.5)

Volume	Invert	Avail.Storage	Storage Description
#1	879.70'	3,565 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
879.70	1,800	0.0	0	0
880.20	1,800	40.0	360	360
883.00	1,800	20.0	1,008	1,368
884.01	2,550	100.0	2,197	3,565

Device	Routing	Invert	Outlet Devices
#1	Primary	879.70'	8.0" Round Culvert L= 116.8' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 879.70' / 879.12' S= 0.0050 ' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	879.70'	0.500 in/hr BIO MEDIA over Surface area
#3	Device 1	883.50'	6.0" W x 6.0" H Vert. WEIR C= 0.600 Limited to weir flow at low heads
#4	Device 1	884.00'	24.0" x 24.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.02 cfs @ 11.90 hrs HW=879.85' (Free Discharge)

1=Culvert (Passes 0.02 cfs of 0.05 cfs potential flow)
2=BIO MEDIA (Exfiltration Controls 0.02 cfs)
3=WEIR (Controls 0.00 cfs)
4=Grate (Controls 0.00 cfs)

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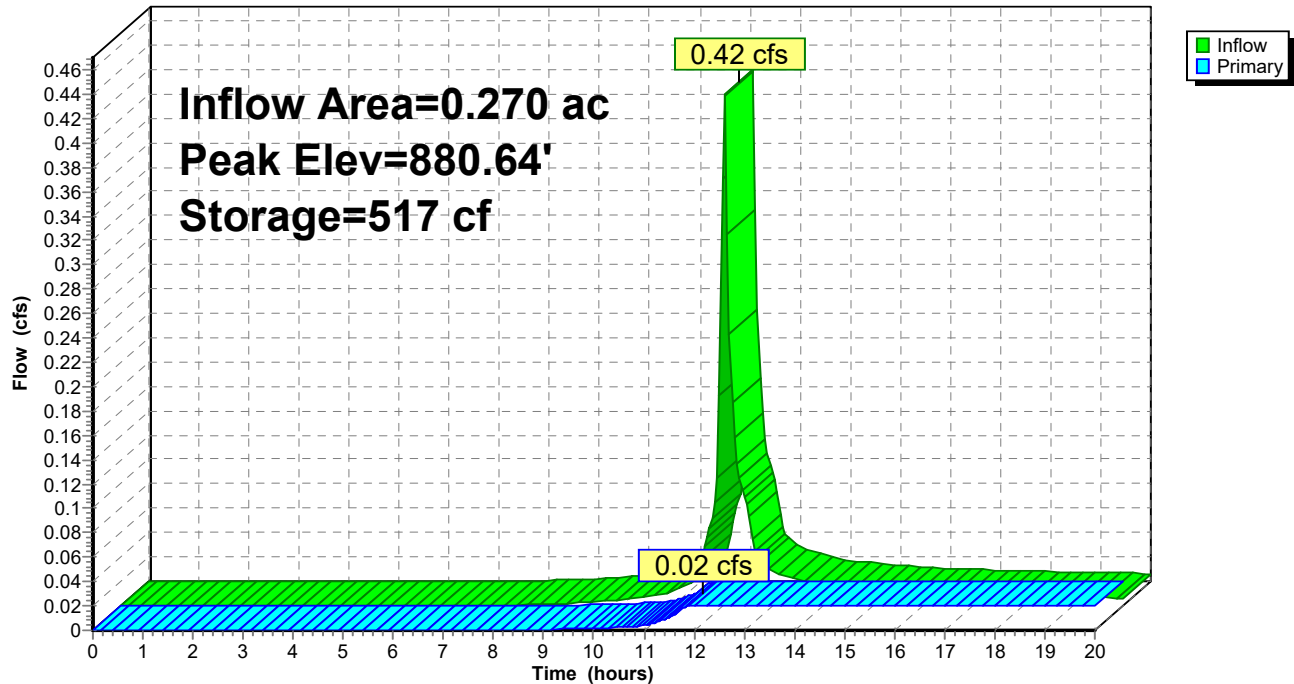
NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 10

Pond 2P: Bio Area 2

Hydrograph



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NY - LeRoy 24-hr S1 1-yr Rainfall=1.85"

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Page 11

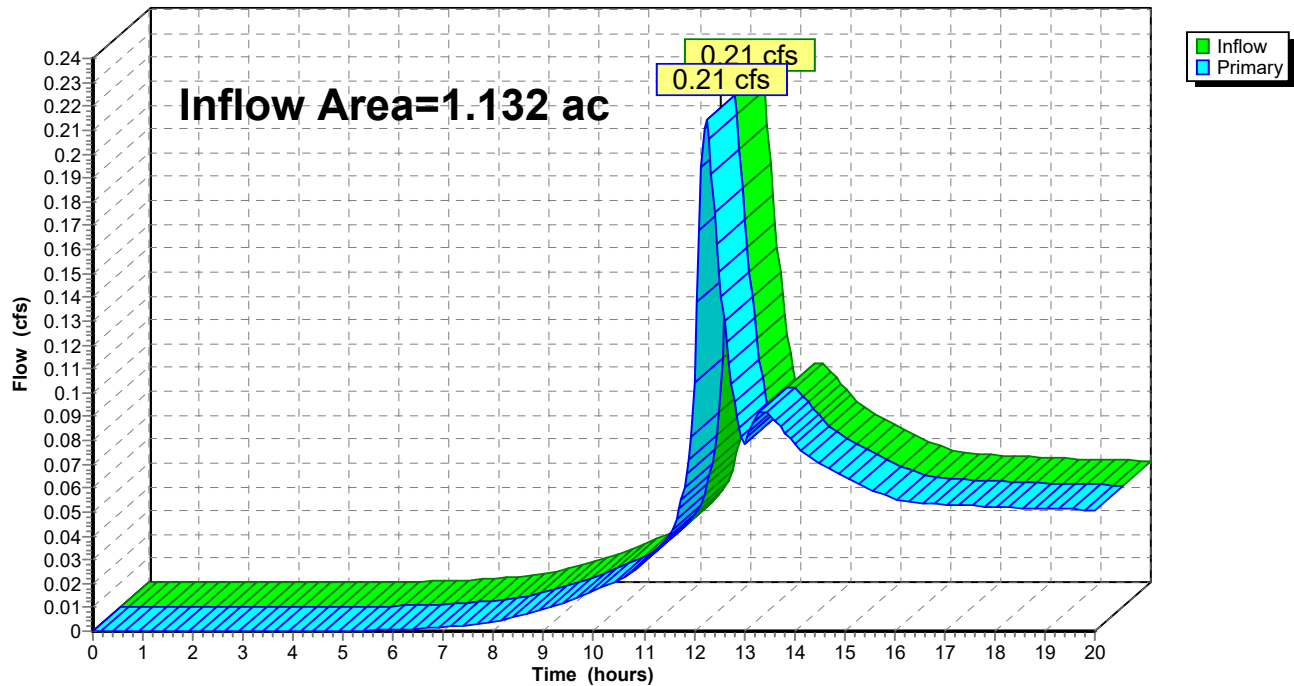
Summary for Link PR: COMBINED PR

Inflow Area = 1.132 ac, 60.07% Impervious, Inflow Depth > 0.58" for 1-yr event
Inflow = 0.21 cfs @ 12.24 hrs, Volume= 0.055 af
Primary = 0.21 cfs @ 12.24 hrs, Volume= 0.055 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs

Link PR: COMBINED PR

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 12

Summary for Subcatchment PR A-1: PR A-1

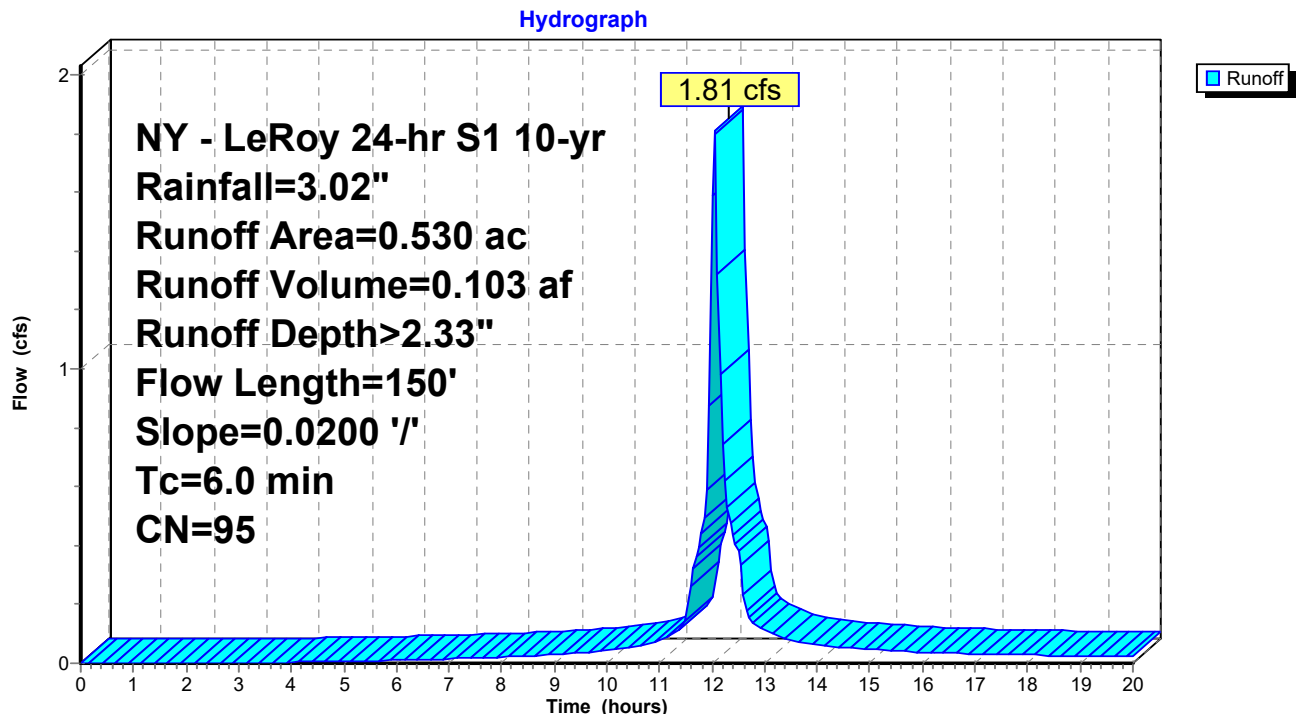
Runoff = 1.81 cfs @ 12.04 hrs, Volume= 0.103 af, Depth> 2.33"
Routed to Pond 1P : Bio Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
* 0.430	98	Impervious
0.100	80	>75% Grass cover, Good, HSG D
0.530	95	Weighted Average
0.100		18.87% Pervious Area
0.430		81.13% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.0	150	0.0200	1.22		Sheet Flow, Sheet Flow
					Smooth surfaces n= 0.011 P2= 2.14"
2.0	150	Total, Increased to minimum Tc = 6.0 min			

Subcatchment PR A-1: PR A-1



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 13

Summary for Subcatchment PR A-3: PR A-2

Runoff = 0.37 cfs @ 12.25 hrs, Volume= 0.032 af, Depth> 1.47"
Routed to Link PR : COMBINED PR

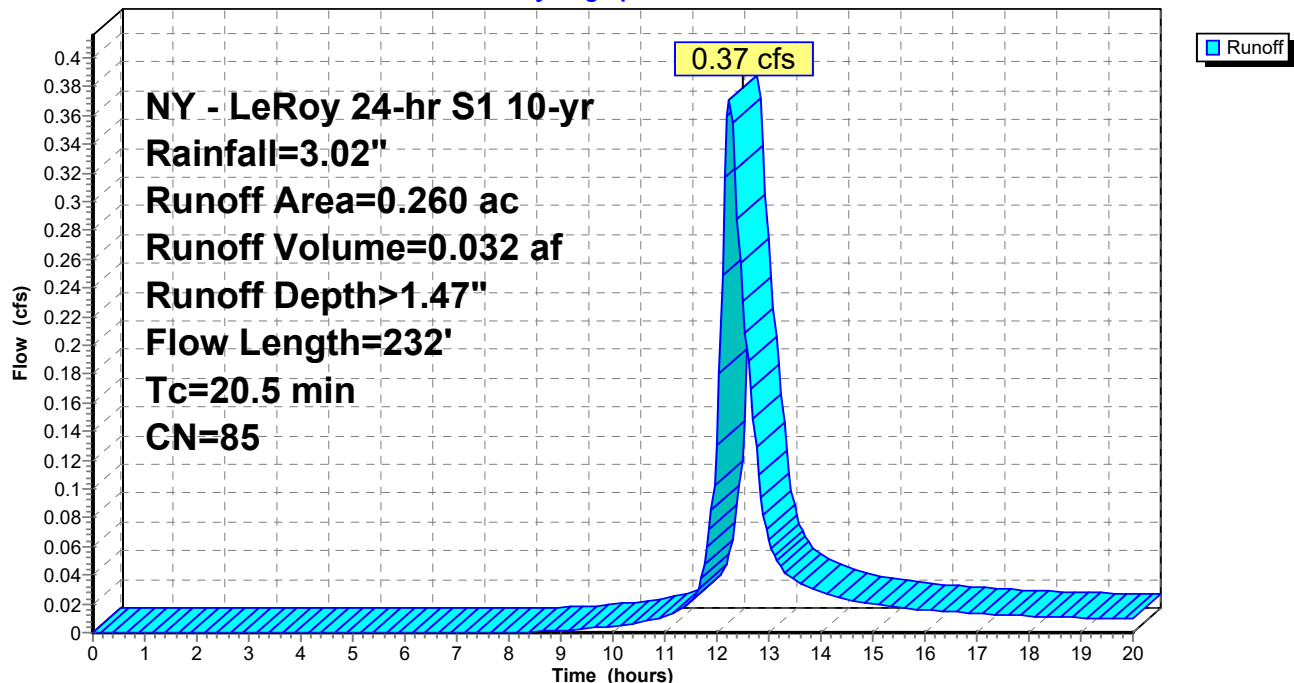
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
* 0.080	98	Impervious
0.090	77	Woods, Good, HSG D
0.090	80	>75% Grass cover, Good, HSG D
0.260	85	Weighted Average
0.180		69.23% Pervious Area
0.080		30.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.5	130	0.0100	0.11		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.5	24	0.0150	0.75		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 2.14"
0.5	78	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
20.5	232	Total			

Subcatchment PR A-3: PR A-2

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 14

Summary for Subcatchment PR B: PR B

Runoff = 0.09 cfs @ 12.07 hrs, Volume= 0.005 af, Depth> 1.04"
Routed to Link PR : COMBINED PR

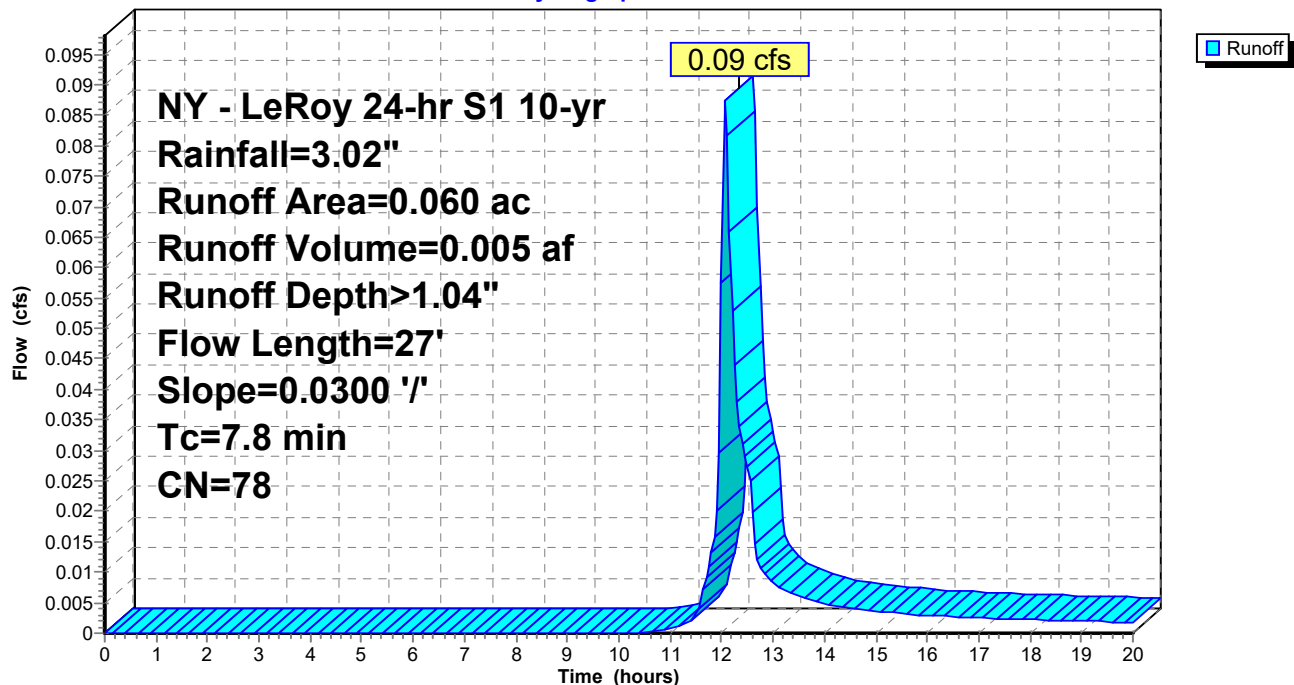
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
0.020	80	>75% Grass cover, Good, HSG D
0.040	77	Woods, Good, HSG D
0.060	78	Weighted Average
0.060		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	27	0.0300	0.06		Sheet Flow, Sheet Flow
					Woods: Light underbrush n= 0.400 P2= 2.14"

Subcatchment PR B: PR B

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 15

Summary for Subcatchment PR C: PR C

Runoff = 0.02 cfs @ 12.05 hrs, Volume= 0.001 af, Depth> 1.10"
Routed to Link PR : COMBINED PR

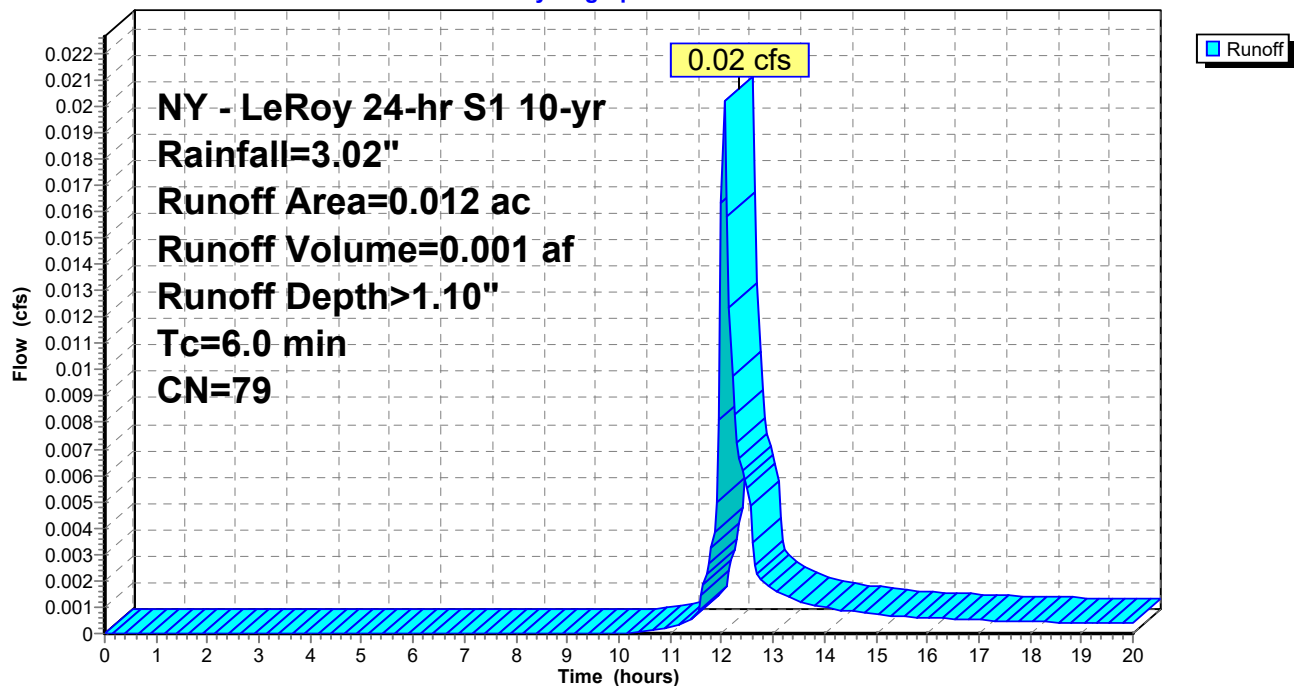
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
0.006	80	>75% Grass cover, Good, HSG D
0.006	77	Woods, Good, HSG D
0.012	79	Weighted Average
0.012		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR C: PR C

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 16

Summary for Subcatchment PR-A-2: PR A-2

Runoff = 0.81 cfs @ 12.04 hrs, Volume= 0.044 af, Depth> 1.95"
Routed to Pond 2P : Bio Area 2

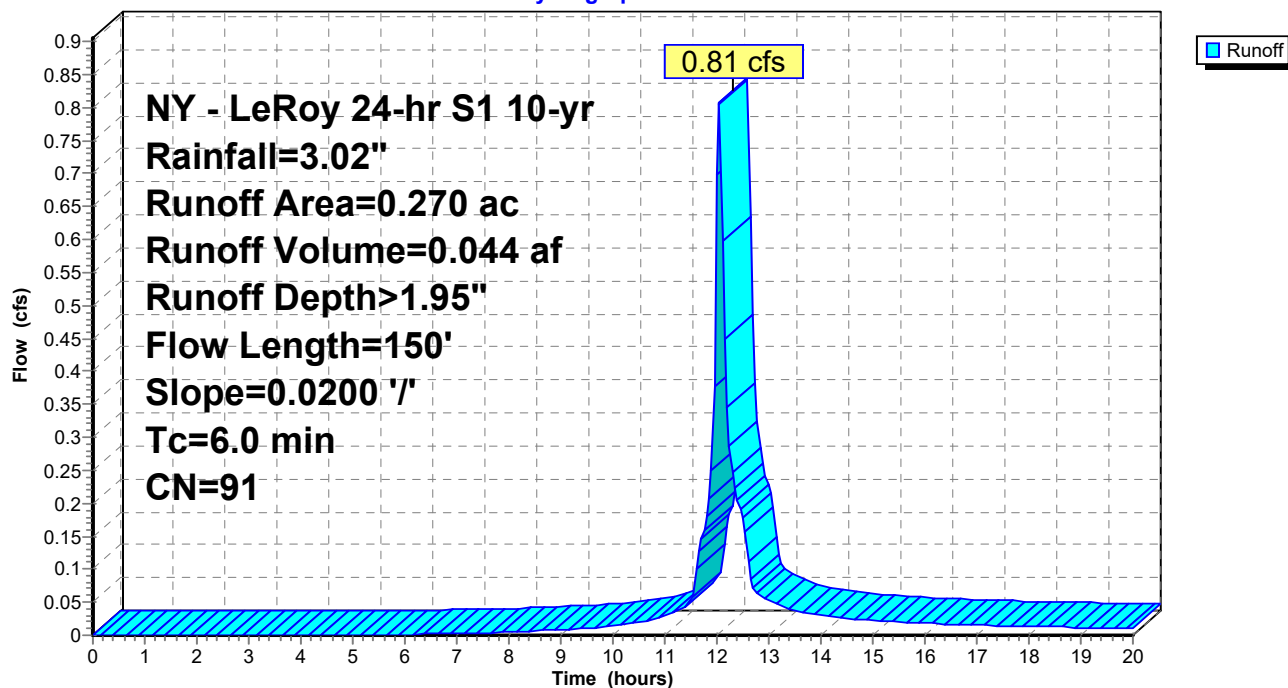
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

Area (ac)	CN	Description
* 0.170	98	Impervious
0.100	80	>75% Grass cover, Good, HSG D
0.270	91	Weighted Average
0.100		37.04% Pervious Area
0.170		62.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.0	150	0.0200	1.22		Sheet Flow, Sheet Flow
					Smooth surfaces n= 0.011 P2= 2.14"
2.0	150	Total, Increased to minimum Tc = 6.0 min			

Subcatchment PR-A-2: PR A-2

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 17

Summary for Pond 1P: Bio Area 1

Inflow Area = 0.530 ac, 81.13% Impervious, Inflow Depth > 2.33" for 10-yr event
Inflow = 1.81 cfs @ 12.04 hrs, Volume= 0.103 af
Outflow = 0.99 cfs @ 12.16 hrs, Volume= 0.070 af, Atten= 45%, Lag= 7.4 min
Primary = 0.99 cfs @ 12.16 hrs, Volume= 0.070 af
Routed to Link PR : COMBINED PR

Routing by Stor-Ind method, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 883.05' @ 12.16 hrs Surf.Area= 2,064 sf Storage= 1,760 cf

Plug-Flow detention time= 104.8 min calculated for 0.069 af (68% of inflow)
Center-of-Mass det. time= 39.7 min (790.0 - 750.2)

Volume	Invert	Avail.Storage	Storage Description	
#1	879.50'	2,828 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
879.50	2,064	0.0	0	0
880.00	2,064	40.0	413	413
883.00	2,064	20.0	1,238	1,651
883.10	2,064	100.0	206	1,858
883.50	2,790	100.0	971	2,828

Device	Routing	Invert	Outlet Devices
#1	Primary	879.50'	8.0" Round Culvert L= 78.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 879.50' / 879.00' S= 0.0064 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	879.50'	0.500 in/hr BIO MEDIA over Surface area
#3	Device 1	882.50'	6.0" W x 6.0" H Vert. WEIR C= 0.600 Limited to weir flow at low heads
#4	Device 1	883.00'	24.0" x 24.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.95 cfs @ 12.16 hrs HW=883.05' (Free Discharge)

- 1=Culvert (Passes 0.95 cfs of 2.16 cfs potential flow)
- 2=BIO MEDIA (Exfiltration Controls 0.02 cfs)
- 3=WEIR (Orifice Controls 0.64 cfs @ 2.54 fps)
- 4=Grate (Weir Controls 0.29 cfs @ 0.73 fps)

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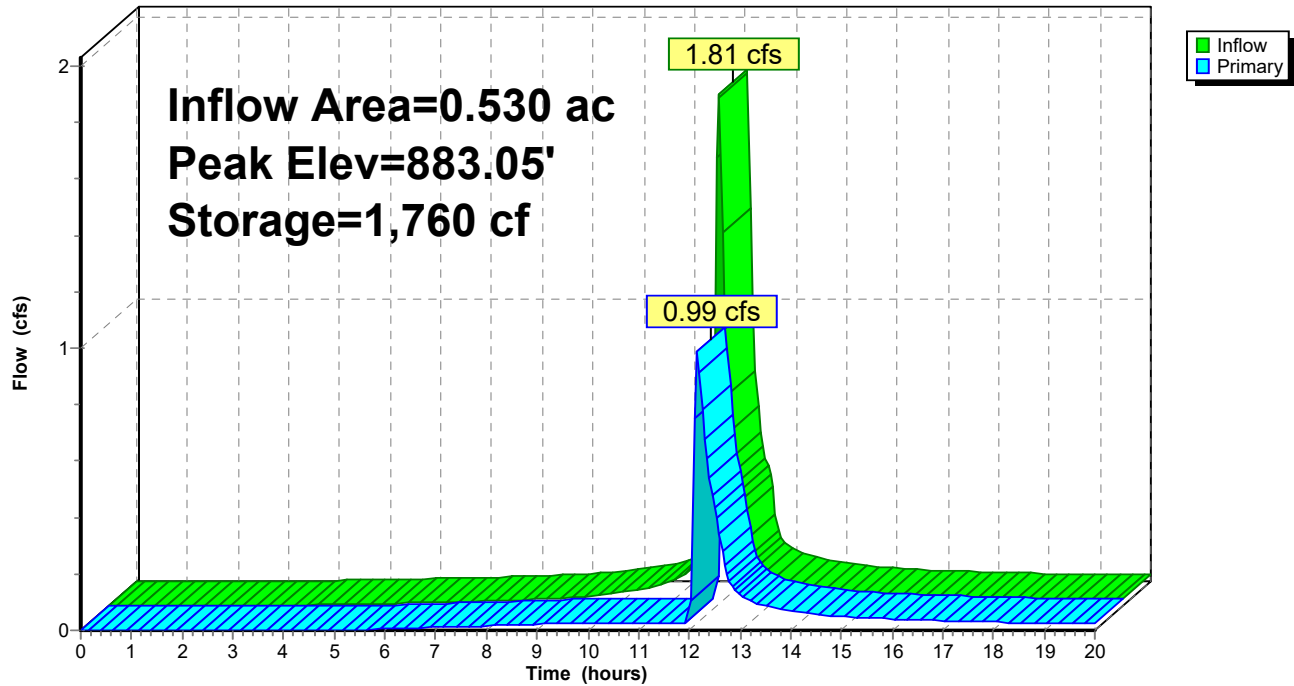
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 18

Pond 1P: Bio Area 1

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 19

Summary for Pond 2P: Bio Area 2

Inflow Area = 0.270 ac, 62.96% Impervious, Inflow Depth > 1.95" for 10-yr event
Inflow = 0.81 cfs @ 12.04 hrs, Volume= 0.044 af
Outflow = 0.02 cfs @ 11.45 hrs, Volume= 0.017 af, Atten= 97%, Lag= 0.0 min
Primary = 0.02 cfs @ 11.45 hrs, Volume= 0.017 af
Routed to Link PR : COMBINED PR

Routing by Stor-Ind method, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 882.71' @ 15.29 hrs Surf.Area= 1,800 sf Storage= 1,264 cf

Plug-Flow detention time= 210.6 min calculated for 0.017 af (40% of inflow)
Center-of-Mass det. time= 124.1 min (893.5 - 769.4)

Volume	Invert	Avail.Storage	Storage Description	
#1	879.70'	3,565 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
879.70	1,800	0.0	0	0
880.20	1,800	40.0	360	360
883.00	1,800	20.0	1,008	1,368
884.01	2,550	100.0	2,197	3,565

Device	Routing	Invert	Outlet Devices
#1	Primary	879.70'	8.0" Round Culvert L= 116.8' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 879.70' / 879.12' S= 0.0050 ' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	879.70'	0.500 in/hr BIO MEDIA over Surface area
#3	Device 1	883.50'	6.0" W x 6.0" H Vert. WEIR C= 0.600 Limited to weir flow at low heads
#4	Device 1	884.00'	24.0" x 24.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.02 cfs @ 11.45 hrs HW=879.83' (Free Discharge)

- 1=Culvert (Passes 0.02 cfs of 0.04 cfs potential flow)
- 2=BIO MEDIA (Exfiltration Controls 0.02 cfs)
- 3=WEIR (Controls 0.00 cfs)
- 4=Grate (Controls 0.00 cfs)

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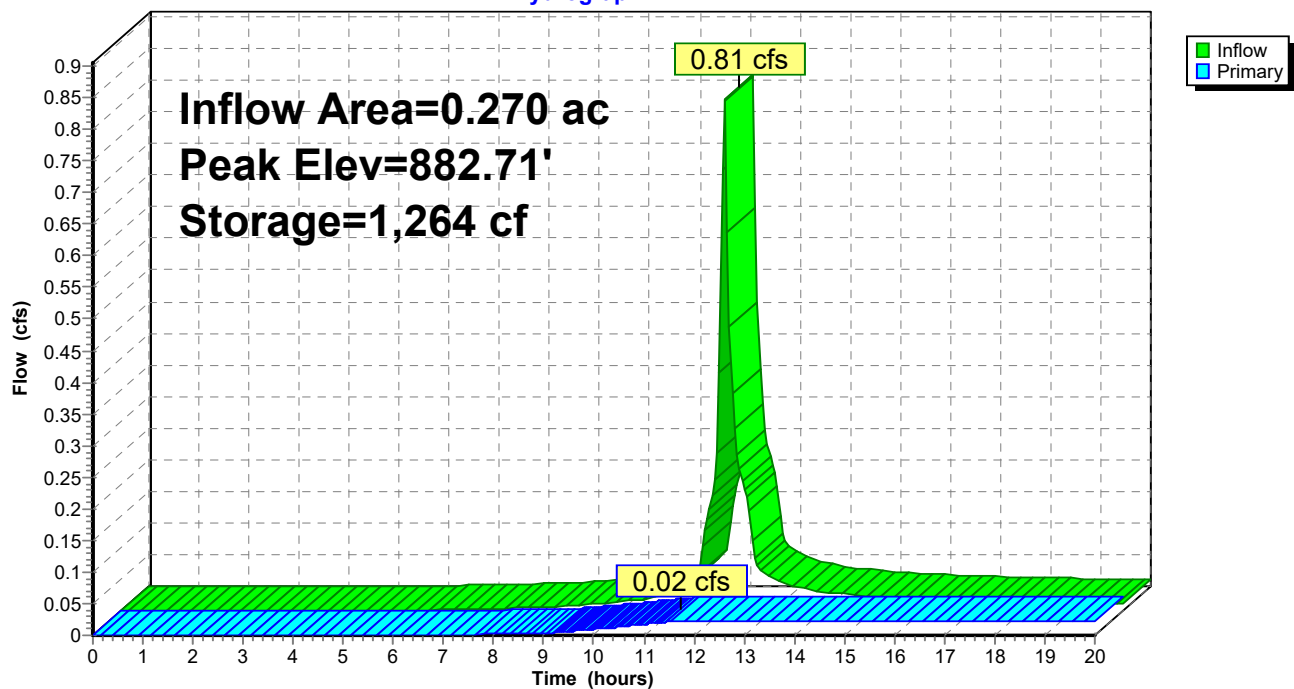
NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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Page 20

Pond 2P: Bio Area 2

Hydrograph



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NY - LeRoy 24-hr S1 10-yr Rainfall=3.02"

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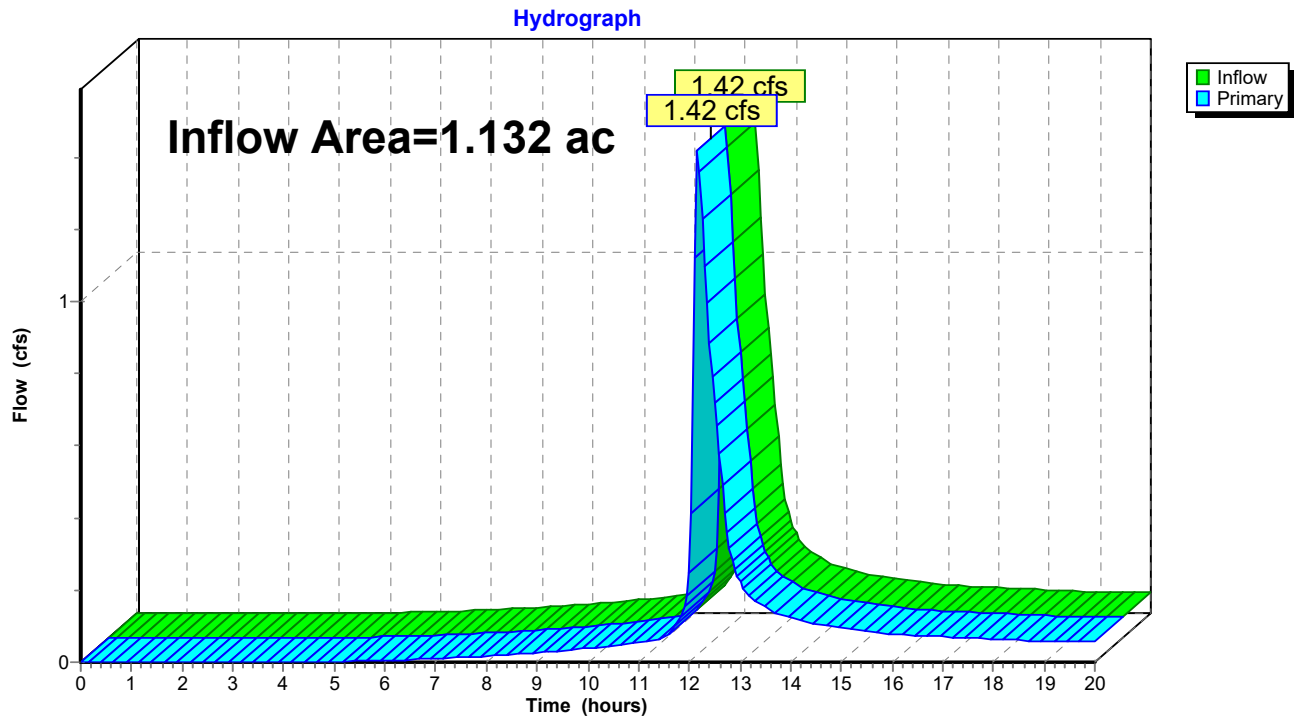
Page 21

Summary for Link PR: COMBINED PR

Inflow Area = 1.132 ac, 60.07% Impervious, Inflow Depth > 1.33" for 10-yr event
Inflow = 1.42 cfs @ 12.17 hrs, Volume= 0.125 af
Primary = 1.42 cfs @ 12.17 hrs, Volume= 0.125 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs

Link PR: COMBINED PR



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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 22

Summary for Subcatchment PR A-1: PR A-1

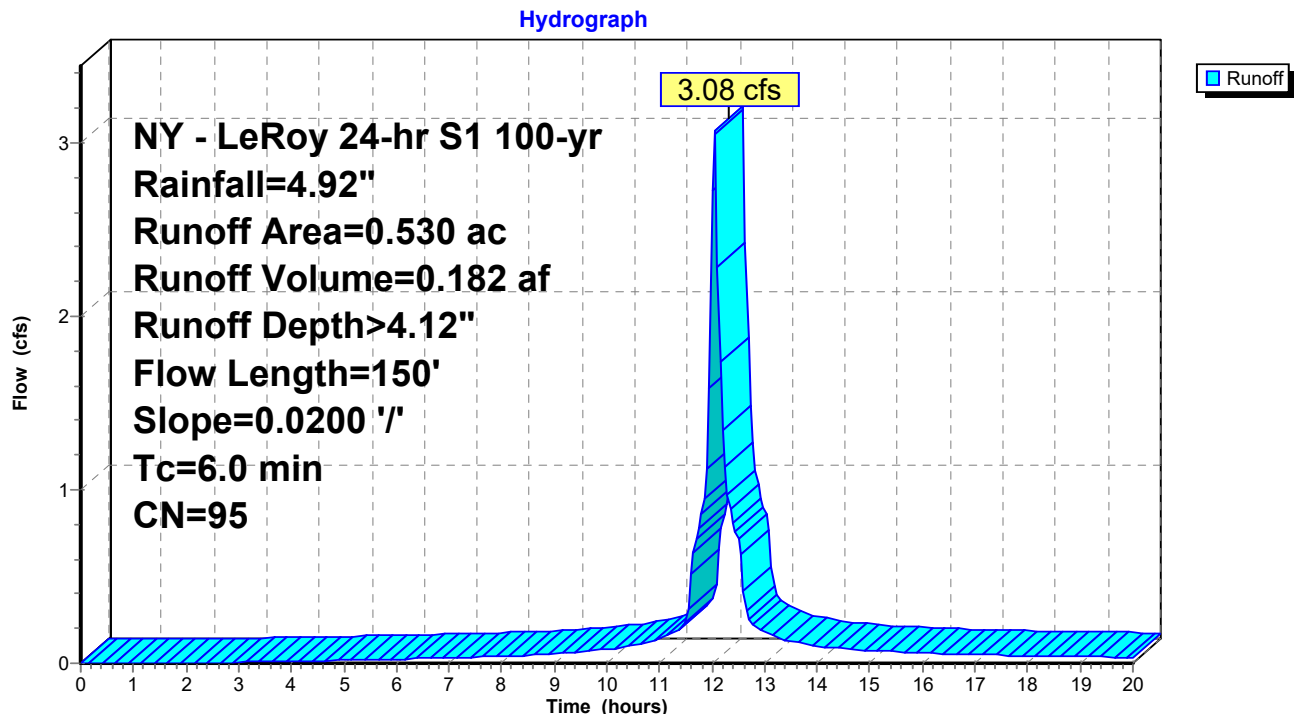
Runoff = 3.08 cfs @ 12.04 hrs, Volume= 0.182 af, Depth> 4.12"
Routed to Pond 1P : Bio Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
* 0.430	98	Impervious
0.100	80	>75% Grass cover, Good, HSG D
0.530	95	Weighted Average
0.100		18.87% Pervious Area
0.430		81.13% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.0	150	0.0200	1.22		Sheet Flow, Sheet Flow
					Smooth surfaces n= 0.011 P2= 2.14"
2.0	150	Total, Increased to minimum Tc = 6.0 min			

Subcatchment PR A-1: PR A-1



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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 23

Summary for Subcatchment PR A-3: PR A-2

Runoff = 0.78 cfs @ 12.24 hrs, Volume= 0.067 af, Depth> 3.08"
Routed to Link PR : COMBINED PR

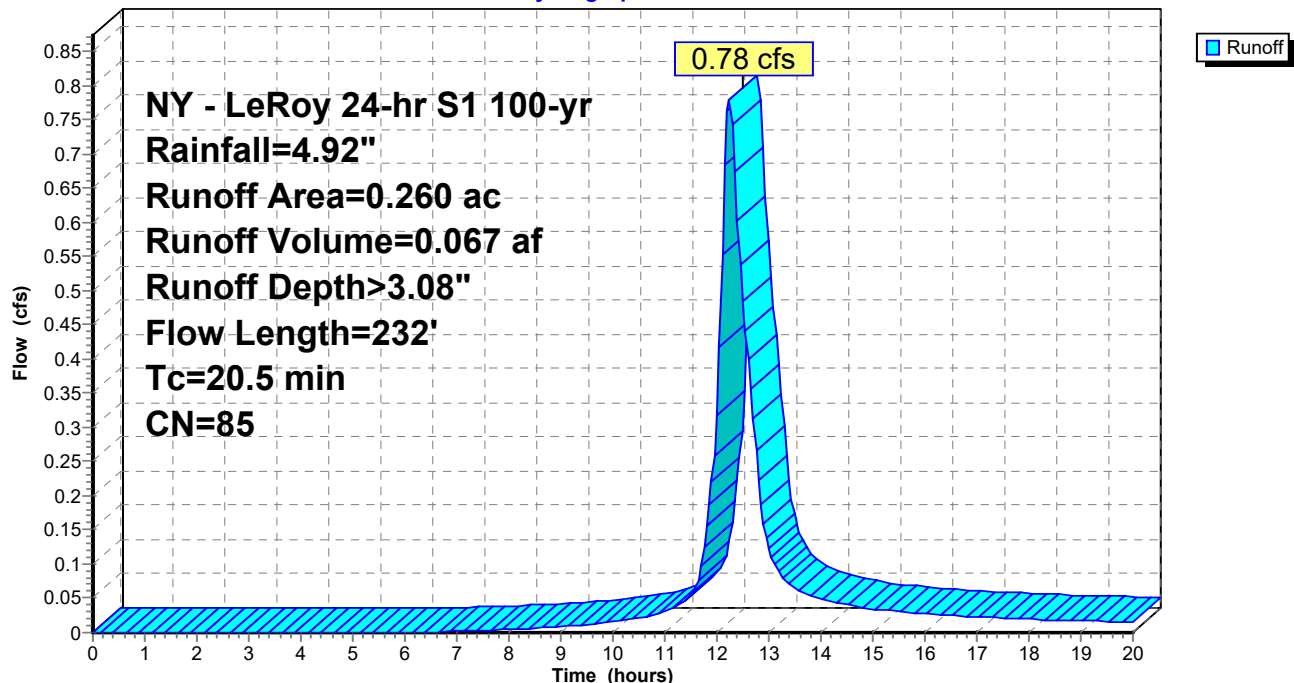
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
* 0.080	98	Impervious
0.090	77	Woods, Good, HSG D
0.090	80	>75% Grass cover, Good, HSG D
0.260	85	Weighted Average
0.180		69.23% Pervious Area
0.080		30.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.5	130	0.0100	0.11		Sheet Flow, Sheet Flow Grass: Short n= 0.150 P2= 2.14"
0.5	24	0.0150	0.75		Sheet Flow, Sheet Flow Smooth surfaces n= 0.011 P2= 2.14"
0.5	78	0.0300	2.79		Shallow Concentrated Flow, Shallow Conc Unpaved Kv= 16.1 fps
20.5	232	Total			

Subcatchment PR A-3: PR A-2

Hydrograph



3036 Drainage

Prepared by BME Associates

HydroCAD® 10.20-7a s/n 09860 © 2025 HydroCAD Software Solutions LLC

NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Printed 9/22/2026

Page 24

Summary for Subcatchment PR B: PR B

Runoff = 0.21 cfs @ 12.07 hrs, Volume= 0.012 af, Depth> 2.46"
Routed to Link PR : COMBINED PR

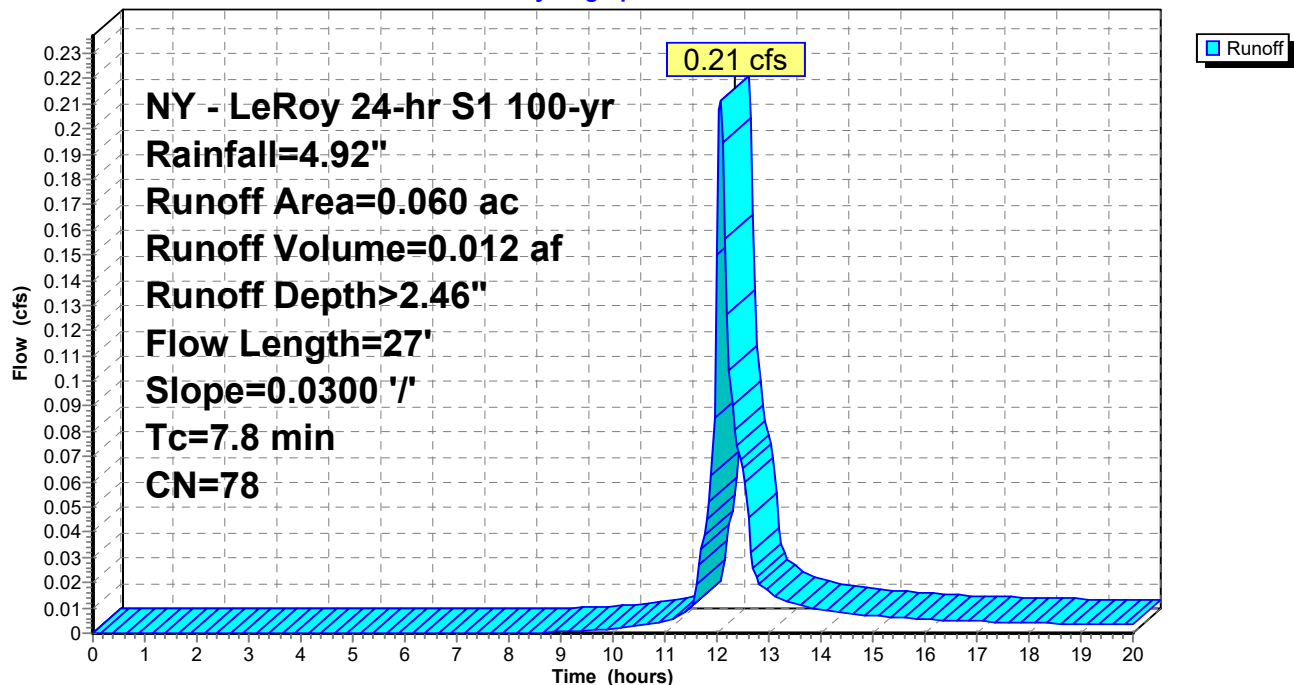
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
0.020	80	>75% Grass cover, Good, HSG D
0.040	77	Woods, Good, HSG D
0.060	78	Weighted Average
0.060		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	27	0.0300	0.06		Sheet Flow, Sheet Flow
					Woods: Light underbrush n= 0.400 P2= 2.14"

Subcatchment PR B: PR B

Hydrograph



3036 Drainage

Prepared by BME Associates

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 25

Summary for Subcatchment PR C: PR C

Runoff = 0.05 cfs @ 12.05 hrs, Volume= 0.003 af, Depth> 2.54"
Routed to Link PR : COMBINED PR

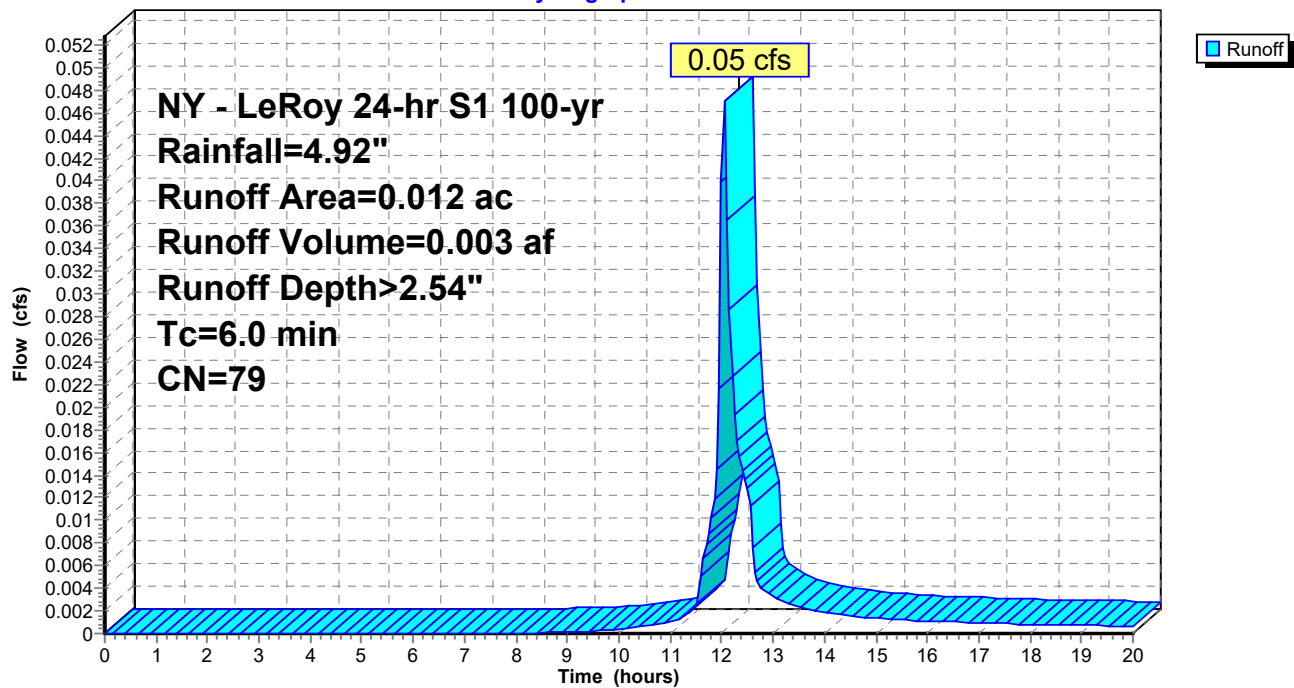
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
0.006	80	>75% Grass cover, Good, HSG D
0.006	77	Woods, Good, HSG D
0.012	79	Weighted Average
0.012		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR C: PR C

Hydrograph



3036 Drainage

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Printed 9/22/2026

Page 26

Summary for Subcatchment PR-A-2: PR A-2

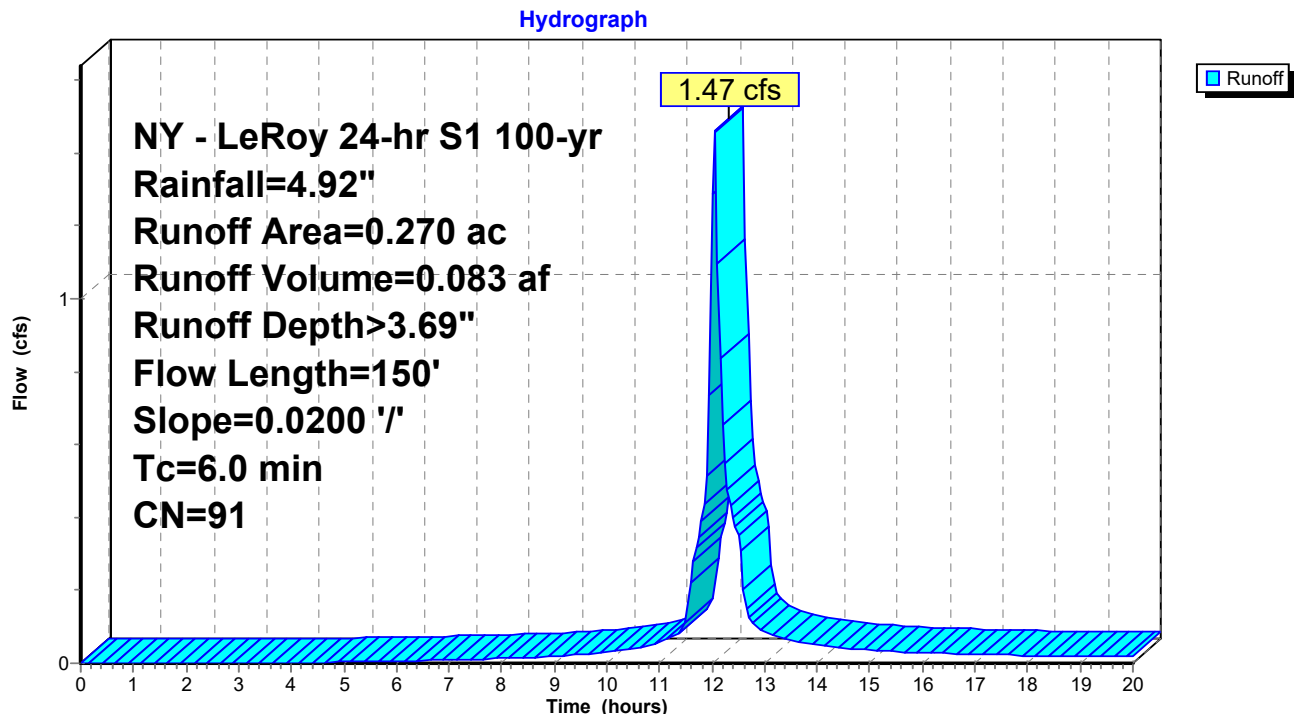
Runoff = 1.47 cfs @ 12.04 hrs, Volume= 0.083 af, Depth> 3.69"
Routed to Pond 2P : Bio Area 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Area (ac)	CN	Description
* 0.170	98	Impervious
0.100	80	>75% Grass cover, Good, HSG D
0.270	91	Weighted Average
0.100		37.04% Pervious Area
0.170		62.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.0	150	0.0200	1.22		Sheet Flow, Sheet Flow
					Smooth surfaces n= 0.011 P2= 2.14"
2.0	150	Total, Increased to minimum Tc = 6.0 min			

Subcatchment PR-A-2: PR A-2



3036 Drainage

Prepared by BME Associates

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

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Page 27

Summary for Pond 1P: Bio Area 1

Inflow Area = 0.530 ac, 81.13% Impervious, Inflow Depth > 4.12" for 100-yr event
Inflow = 3.08 cfs @ 12.04 hrs, Volume= 0.182 af
Outflow = 2.21 cfs @ 12.11 hrs, Volume= 0.149 af, Atten= 28%, Lag= 4.2 min
Primary = 2.21 cfs @ 12.11 hrs, Volume= 0.149 af
Routed to Link PR : COMBINED PR

Routing by Stor-Ind method, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 883.23' @ 12.11 hrs Surf.Area= 2,302 sf Storage= 2,144 cf

Plug-Flow detention time= 78.0 min calculated for 0.148 af (81% of inflow)
Center-of-Mass det. time= 29.8 min (767.9 - 738.0)

Volume	Invert	Avail.Storage	Storage Description	
#1	879.50'	2,828 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
879.50	2,064	0.0	0	0
880.00	2,064	40.0	413	413
883.00	2,064	20.0	1,238	1,651
883.10	2,064	100.0	206	1,858
883.50	2,790	100.0	971	2,828

Device	Routing	Invert	Outlet Devices
#1	Primary	879.50'	8.0" Round Culvert L= 78.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 879.50' / 879.00' S= 0.0064 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	879.50'	0.500 in/hr BIO MEDIA over Surface area
#3	Device 1	882.50'	6.0" W x 6.0" H Vert. WEIR C= 0.600 Limited to weir flow at low heads
#4	Device 1	883.00'	24.0" x 24.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.21 cfs @ 12.11 hrs HW=883.23' (Free Discharge)

- 1=Culvert (Barrel Controls 2.21 cfs @ 6.34 fps)
- 2=BIO MEDIA (Passes < 0.03 cfs potential flow)
- 3=WEIR (Passes < 0.82 cfs potential flow)
- 4=Grate (Passes < 2.83 cfs potential flow)

3036 Drainage

Prepared by BME Associates

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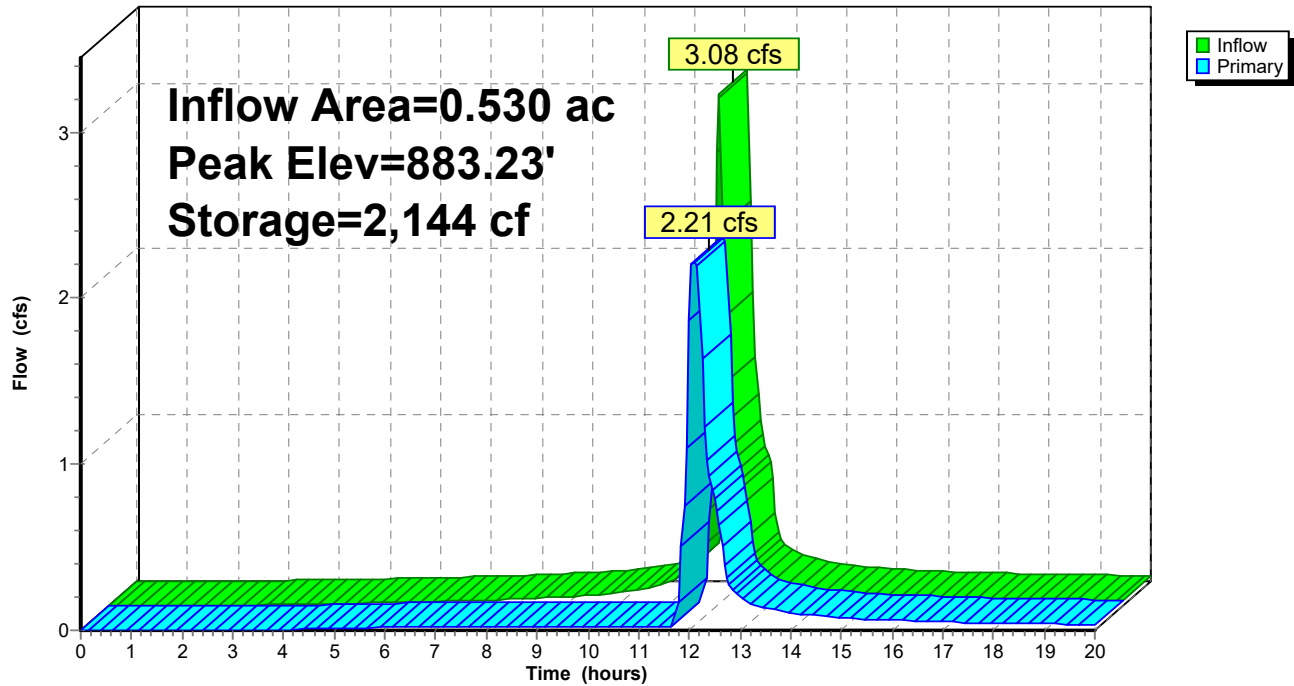
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Printed 9/22/2026

Page 28

Pond 1P: Bio Area 1

Hydrograph



3036 Drainage

Prepared by BME Associates

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Printed 9/22/2026

Page 29

Summary for Pond 2P: Bio Area 2

Inflow Area = 0.270 ac, 62.96% Impervious, Inflow Depth > 3.69" for 100-yr event
Inflow = 1.47 cfs @ 12.04 hrs, Volume= 0.083 af
Outflow = 0.05 cfs @ 14.01 hrs, Volume= 0.030 af, Atten= 97%, Lag= 118.0 min
Primary = 0.05 cfs @ 14.01 hrs, Volume= 0.030 af
Routed to Link PR : COMBINED PR

Routing by Stor-Ind method, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 883.56' @ 14.01 hrs Surf.Area= 2,215 sf Storage= 2,490 cf

Plug-Flow detention time= 202.7 min calculated for 0.029 af (35% of inflow)
Center-of-Mass det. time= 108.3 min (862.6 - 754.2)

Volume	Invert	Avail.Storage	Storage Description	
#1	879.70'	3,565 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
879.70	1,800	0.0	0	0
880.20	1,800	40.0	360	360
883.00	1,800	20.0	1,008	1,368
884.01	2,550	100.0	2,197	3,565

Device	Routing	Invert	Outlet Devices
#1	Primary	879.70'	8.0" Round Culvert L= 116.8' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 879.70' / 879.12' S= 0.0050 ' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	879.70'	0.500 in/hr BIO MEDIA over Surface area
#3	Device 1	883.50'	6.0" W x 6.0" H Vert. WEIR C= 0.600 Limited to weir flow at low heads
#4	Device 1	884.00'	24.0" x 24.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.05 cfs @ 14.01 hrs HW=883.56' (Free Discharge)

- 1=Culvert (Passes 0.05 cfs of 1.95 cfs potential flow)
- 2=BIO MEDIA (Exfiltration Controls 0.03 cfs)
- 3=WEIR (Orifice Controls 0.02 cfs @ 0.78 fps)
- 4=Grate (Controls 0.00 cfs)

3036 Drainage

Prepared by BME Associates

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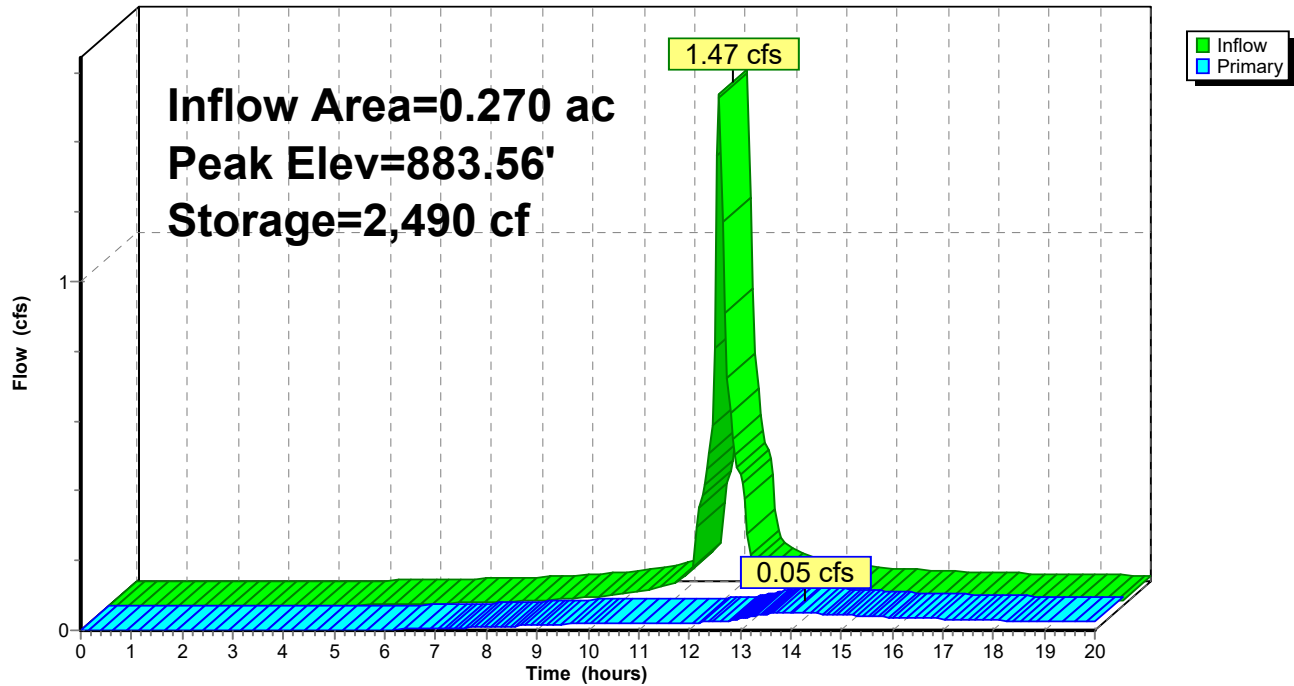
NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Printed 9/22/2026

Page 30

Pond 2P: Bio Area 2

Hydrograph



3036 Drainage

Prepared by BME Associates

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NY - LeRoy 24-hr S1 100-yr Rainfall=4.92"

Printed 9/22/2026

Page 31

Summary for Link PR: COMBINED PR

Inflow Area = 1.132 ac, 60.07% Impervious, Inflow Depth > 2.75" for 100-yr event
Inflow = 3.17 cfs @ 12.18 hrs, Volume= 0.260 af
Primary = 3.17 cfs @ 12.18 hrs, Volume= 0.260 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-20.00 hrs, dt= 0.05 hrs

Link PR: COMBINED PR

Hydrograph

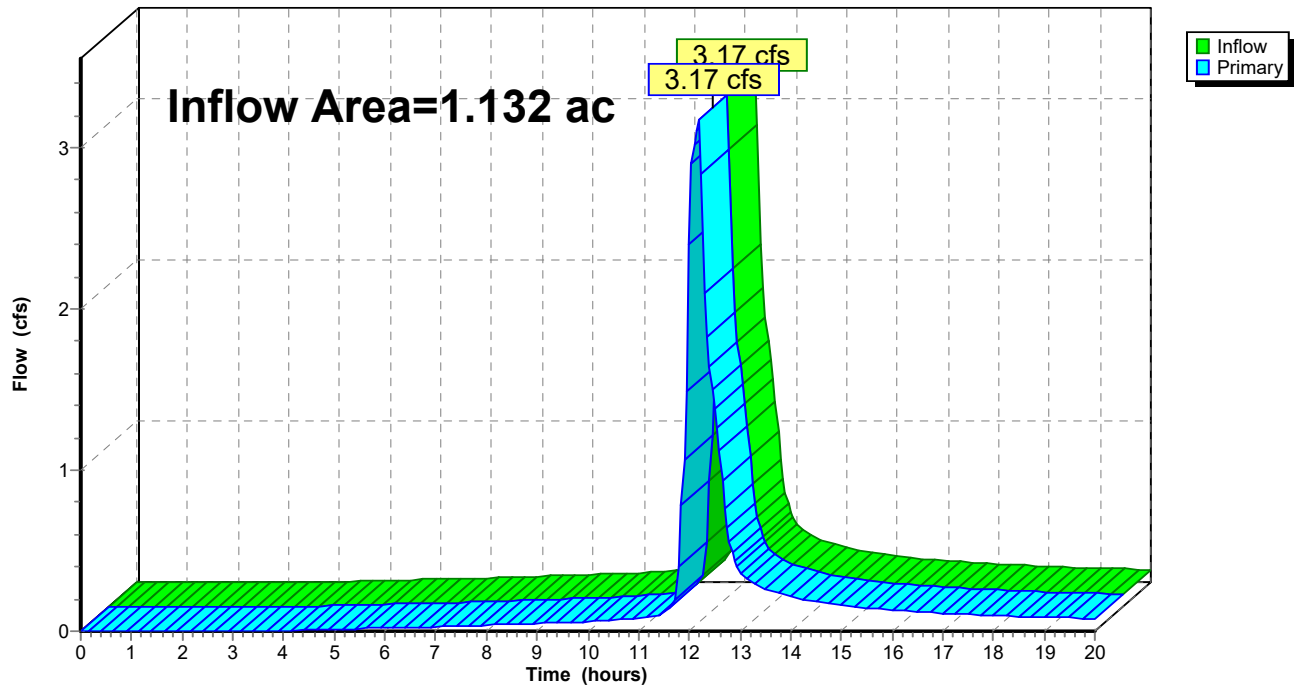


FIGURE 02

Hydrologic Soil Group—Genesee County, New York



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points

 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Genesee County, New York
 Survey Area Data: Version 26, Sep 2, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 13, 2023—May 27, 2023

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
KnA	Kendaia silt loam, 0 to 3 percent slopes	B/D	2.0	98.1%
OnB	Ontario loam, 3 to 8 percent slopes	B	0.0	0.3%
OvB	Ovid silt loam, 3 to 8 percent slopes	C/D	0.0	1.6%
Totals for Area of Interest			2.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

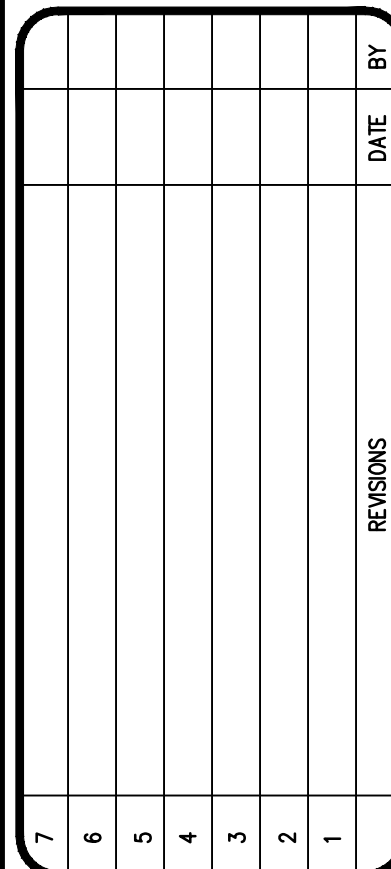
Tie-break Rule: Higher

Filtration Bioretention (F-5)

Design Point:	1						
Enter Site Data For Drainage Area to be Treated by Practice							
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (cf)	Precipitation (in)	Description
1	0.53	0.43	81	0.78	1,501	1.00	Filtration Bioretention
Design Criteria							
Enter underlying soil infiltration rate (based on geotechnical testing, refer to Appendix D)			0.1	Underdrains required			
Is the contributing area to the practice a stormwater hotspot?			No				
Is the practice the first in series for treatment of a Level 1 (Infiltration Restricted) hotspot?			No				
Is contributing area greater than max. contributing area?			No				
Enter depth to seasonal high water table (ft)			5				
Enter depth to bedrock (ft)			17				
Is pretreatment provided, in conformance with Section 6.4.3.1?			Yes				
Enter average height of ponding (ft)			0.5				
Enter depth of surface layer (inches)			3				
Enter depth of filter media (ft)			2.5				
Enter depth of drainage layer (inches)			10				
Enter slope of maintenance access (%)			2				
Enter width of maintenance access (ft)			12				
Sizing Criteria							
				Value	Units	Notes	
Permeability Flow Rate			k	1	ft/day		
Filter Time			tf	2	days		
Required Filter Area			Af	625	sf		
Enter Provided Filter Area			Af	2064	sf		
Recalculated Water Quality Volume (based on provided filter area)			WQv calc	4953.6	cf		
Calculate Runoff Reduction							
RRv Provided		1,501	cf				
WQv Treated		0	cf	This is the portion of the WQv that is not reduced in the practice.			

Filtration Bioretention (F-5)

Design Point:	1						
Enter Site Data For Drainage Area to be Treated by Practice							
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (cf)	Precipitation (in)	Description
2	0.27	0.17	63	0.62	604	1.00	Filtration
Design Criteria							
Enter underlying soil infiltration rate (based on geotechnical testing, refer to Appendix D)			0.1	Underdrains required			
Is the contributing area to the practice a stormwater hotspot?			No				
Is the practice the first in series for treatment of a Level 1 (Infiltration Restricted) hotspot?			No				
Is contributing area greater than max. contributing area?			No				
Enter depth to seasonal high water table (ft)			5				
Enter depth to bedrock (ft)			16				
Is pretreatment provided, in conformance with Section 6.4.3.1?			Yes				
Enter average height of ponding (ft)			0.5				
Enter depth of surface layer (inches)			3				
Enter depth of filter media (ft)			2.5				
Enter depth of drainage layer (inches)			10				
Enter slope of maintenance access (%)			2				
Enter width of maintenance access (ft)			12				
Sizing Criteria							
				Value	Units	Notes	
Permeability Flow Rate			k	1	ft/day		
Filter Time			tf	2	days		
Required Filter Area			Af	252	sf		
Enter Provided Filter Area			Af	1800	sf		
Recalculated Water Quality Volume (based on provided filter area)			WQv calc	4320	cf		
Calculate Runoff Reduction							
RRv Provided	604	cf					
WQv Treated	0	cf	This is the portion of the WQv that is not reduced in the practice.				



BME ASSOCIATES
ENGINEERS • SURVEYORS • LANDSCAPE ARCHITECTS

800 PARKER HILL DRIVE, SUITE 300
ROCHESTER, NEW YORK 14625

WWW.BMEPC.COM
585-377-7360

PROJECT	LEROY TACO BELL
LOCATION	TOWN/VILLAGE OF LEROY, GENESSEE COUNTY, NEW YORK STATE
CLIENT	INDUS HOSPITALITY GROUP 905 PANORAMA TRAIL S ROCHESTER, NEW YORK, 14625
DRAWING TITLE	STORM SEWER MAP

PROJECT MANAGER	
P. VARS	
PROJECT ENGINEER	
R. SPURR	
DRAWN BY	
X	
SCALE	DATE ISSUED
1" = 20'	AUGUST 6, 2026
PROJECT NO.	
3036	
DRAWING NO.	
FIGURE 05	

Sheet 1 of 1

[illegible]

CALCULATIONS:

Project: Rip Rap Calculations

Project No.: 3036

Date: 9/22/2026

By: RRS

Sheet 1 OF 1

DESIGNATION

Do-DIA. OF PIPE

DISCHARGE

d50 RIP-RAP SIZE

La-LENGTH OF APRON

W=Do+La

SY OF RIP RAP

MIN BLANKET THICKNESS

AVE WEIGHT

D

18 in

2.74 cfs

0.5 ft

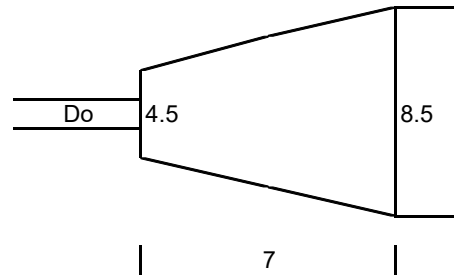
7.0 ft

8.5 ft

5 sy

14 in

50 lbs



NOTES:

Appendix C

Background Information



Delineated Wetland

Perennial Stream

Site Boundary

● UP-1
Data Sampling Point

Date: 06/19/2026

Project No. 3036

BME
ASSOCIATES

LeRoy Taco Bell

USACE Wetland Delineation Map
Town of LeRoy, Genesee County, New York
P:\3036\Drawings\Final\3036 Wetland Figures Base.dwg

Scale:
1"=125'

FIGURE 6



July 29, 2026

Jeremy Deuel
Deuel Archaeology & CRM
PO Box 51
Attica, NY 14011

Re: DEC
LeRoy Taco Bell
7185 W Main Rd, LeRoy, NY 14482
26PR06073

Dear Jeremy Deuel:

Thank you for requesting the comments of the Division for Historic Preservation of the Office of Parks, Recreation and Historic Preservation (OPRHP). We have reviewed the submitted materials in accordance with the New York State Historic Preservation Act of 1980 (Section 14.09 of the New York State Parks, Recreation and Historic Preservation Law). These comments are those of the Division for Historic Preservation and relate only to Historic/Cultural resources. They do not include potential environmental impacts to New York State Parkland that may be involved in or near your project.

The proposed project area is adjacent to the Champion House (7175 West Main Rd). Constructed 1831-1835 for Le Roy merchant John Champion, it is eligible for its cobblestone construction and as an excellent example of a Greek Revival Style house. From the project received on 7/24/2026, we note that new commercial construction is proposed in the lot adjacent to the Champion House. We further note that the house at 7185 West Main Rd is not eligible for listing and we have no concerns about its proposed removal. Based on this review, it is OPRHP's opinion that the proposed project will have No Adverse Impact on properties, including archaeological and/or historic resources, listed in or eligible for the National Registers of Historic Places; provided the following conditions can be met:

- In order to minimize visual impacts to the historic Champion House property, a vegetation buffer shall be maintained between the proposed new construction and the historic cobblestone house located at 7175 West Main Rd.

If you have any questions, I am best reached by email.

Sincerely,

Derek Rohde
Historic Site Restoration Coordinator
518-275-5745 | Derek.Rohde@parks.ny.gov



U.S. Fish and Wildlife Service

National Wetlands Inventory

Leroy Taco Bell



U.S. Fish and Wildlife Service, National Standards and Support Team,
wetlands_team@fws.gov

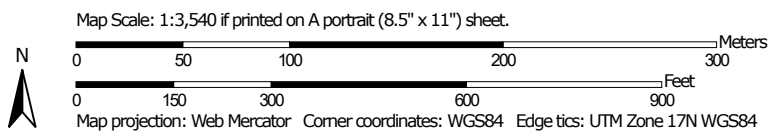
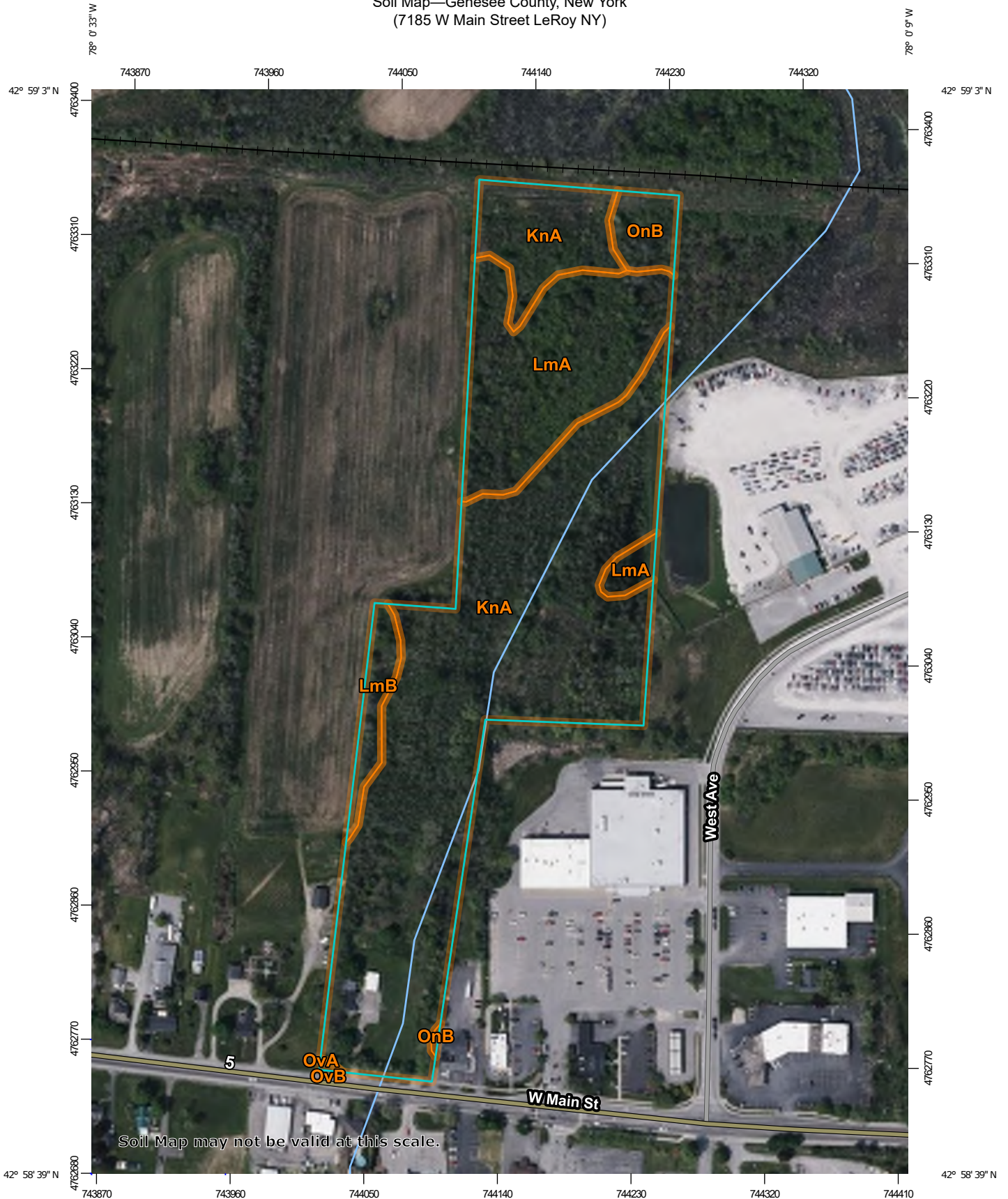
June 10, 2026

Wetlands

	Estuarine and Marine Deepwater		Freshwater Emergent Wetland		Lake
	Estuarine and Marine Wetland		Freshwater Forested/Shrub Wetland		Other
			Freshwater Pond		Riverine

This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the Wetlands Mapper web site.

Soil Map—Genesee County, New York
(7185 W Main Street LeRoy NY)



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Genesee County, New York

Survey Area Data: Version 26, Sep 2, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 13, 2023—May 27, 2023

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

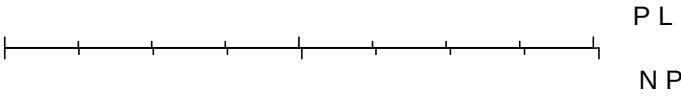
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
KnA	Kendaia silt loam, 0 to 3 percent slopes	12.7	72.0%
LmA	Lima silt loam, 0 to 3 percent slopes	3.8	21.5%
LmB	Lima silt loam, 3 to 8 percent slopes	0.6	3.2%
OnB	Ontario loam, 3 to 8 percent slopes	0.6	3.2%
OvA	Ovid silt loam, 0 to 3 percent slopes	0.0	0.0%
OvB	Ovid silt loam, 3 to 8 percent slopes	0.0	0.1%
Totals for Area of Interest		17.7	100.0%



/HUR\ 7DFR %HOO



-XQH



1<6 ,76 *HRVSDWLDO 6HUYLEFHV 1HZ <RUN 6WD

1<6 'HSDUWPHQW RI (QYLURQPHQV
1RW D OHJDO GRF

Appendix D

Traffic Correspondence

Rebecca Spurr

From: Miller, Thomas C (DOT) <Thomas.Miller2@dot.ny.gov>
Sent: Friday, June 12, 2026 10:58 AM
To: Rebecca Spurr
Cc: Oravec, Matthew C (DOT); Willey, Justin G. (DOT); jott@indushg.com; Peter Vars
Subject: RE: NYS Rt-5 (Genesee County) Potential Development [Filed 12 Jun 2026 15:01]

Categories: Filed by Mail Manager

Good morning,

NYSDOT has reviewed the concept plan for the proposed quick serve restaurant at #7185 W Main Road (NYS Route 5) in LeRoy. We concur with the proposed site layout to use the existing curb cut at the east side of the property and remove the driveway at the west site. The proposed site driveway should have one enter lane and one exit lane, and the existing apron at Route 5 should be modified as needed (width, radius or taper) to conform to NYSDOT's driveway standards in Standard Sheet 608-03. Additional traffic analysis is not required.

Highway Work Permits will be required for all work within the Route 5 right-of-way including driveway, sidewalk, grading, utility connections, etc. Permit applications may be initiated through our online system PermiTrack.

Thank you for the opportunity to review,

Tom

THOMAS MILLER, P.E., PTOE

Regional Permit Engineer

New York State Department of Transportation

1530 Jefferson Road, Rochester, NY 14623

(585) 272-3472 | Thomas.Miller2@dot.ny.gov

From: Rebecca Spurr <rspurr@bmepc.com>
Sent: Thursday, June 11, 2026 4:35 PM
To: Oravec, Matthew C (DOT) <Matthew.Oravec@dot.ny.gov>; Willey, Justin G. (DOT) <Justin.Willey@dot.ny.gov>
Cc: John Ott (jott@indushg.com) <jott@indushg.com>; Peter Vars <pvars@bmepc.com>
Subject: NYS Rt-5 (Genesee County) Potential Development

Attention: This email came from an external source. Do not open attachments or click on links from unknown senders or unexpected emails.

Good afternoon,

We are working with Indus Hospitality Group on a potential new development along the north side of NYS RT-5 (7185 W. Main Road). The parcel is uniquely within both the Village and Town of LeRoy. We are reaching out to the NYSDOT in the conceptual stages of this potential development to receive initial feedback on the development.

The developer is proposing a +/-2,235 sf drive-thru restaurant, average 8 car stack and +/- 20 parking spaces on site. The drive-thru lane will be designed to provide the min. number of car stacking per the brand and allow the cars within peak times for the restaurant to not backup onto Rt-5. Parking on site will meet code requirements.

This parcel appears to have 2 existing full-service access drives. The easternmost existing drive is 26' wide and the westernmost existing drive appears the same with wider turning radii. The developer is proposing to utilize the easternmost access drive as a full-service access drive. A modification to widen the drive may be required. In addition, a utility NYSDOT highway work permit will likely be required.

The proposed trip generation for the proposed restaurant has been established per the methods of the Institute of Transportation Engineers (ITE) Trip Generation Manual. Table 1 below indicates the total site generated trips for the AM and PM peak hours for the proposed use:

Table 1: Site Trip Generations

Land Use	ITE Group	Size	Seats	AM Peak Hour		PM Peak Hour	
				Enter	Exit	Enter	Exit
Fast Food Restaurant with Drive-Through	934	+/-2,235 sf	40	29	25	33	32
Pass-by Rates (30% AM, 40% PM) *				-8	-7	-13	-12
Total Primary Trips				21	18	20	20

*The service land use type is expected to generate pass-by trips. Pass-by rates were based on the ITE Trip Generation Handbook and were assigned as 30% during the weekday morning peak hour, and 40% during the weekday evening and Saturday midday peak hours.

As noted in the table, not all these driveway volumes are new, but instead a portion of the proposed volume is reduced considering pass-by credits. The proposed project is expected to generate approximately 21 entering / 18 exiting vehicle trips during the AM peak hour and 20 entering / 20 exiting vehicle trips during the PM peak hour. For purposes of the environmental review of the proposed project pursuant to the State Environmental Quality Review Act (SEQRA), based on the proposed generated trips it appears the proposed project will not result in any potentially significant adverse traffic impacts to the surrounding road network.

We would request the NYSDOT's review of the above information and potential development and offer any initial feedback and if any additional traffic analysis or studies would be required. The developer is looking to initiate the site plan review process with the Town/Village this month.

Please contact our office if you have any questions.

Thank you,

Rebecca R. Spurr, P.E.

Project Engineer

BME ASSOCIATES
ENGINEERS • SURVEYORS • LANDSCAPE ARCHITECTS

800 Parker Hill Drive, Suite 300

Rochester, NY 14625

Phone: (585) 377-7360 x108

www.bmepc.com



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N/F
JONATHAN EHRMENTRAUT
7175 WEST MAIN ROAD
T.A. No. 24-1-27

N/F
AGOSTINO BALASAMO
7185 WEST MAIN ROAD
T.A. No. 24-1-26

N/F
AGOSTINO BALASAMO
7185 WEST MAIN ROAD
T.A. No. 3-1-2

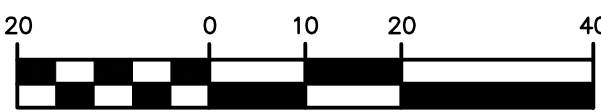
N/F
NO FINER PROPERTIES, LLC
140 WEST MAIN STREET
T.A. No. 3-1-3.11

LEGEND

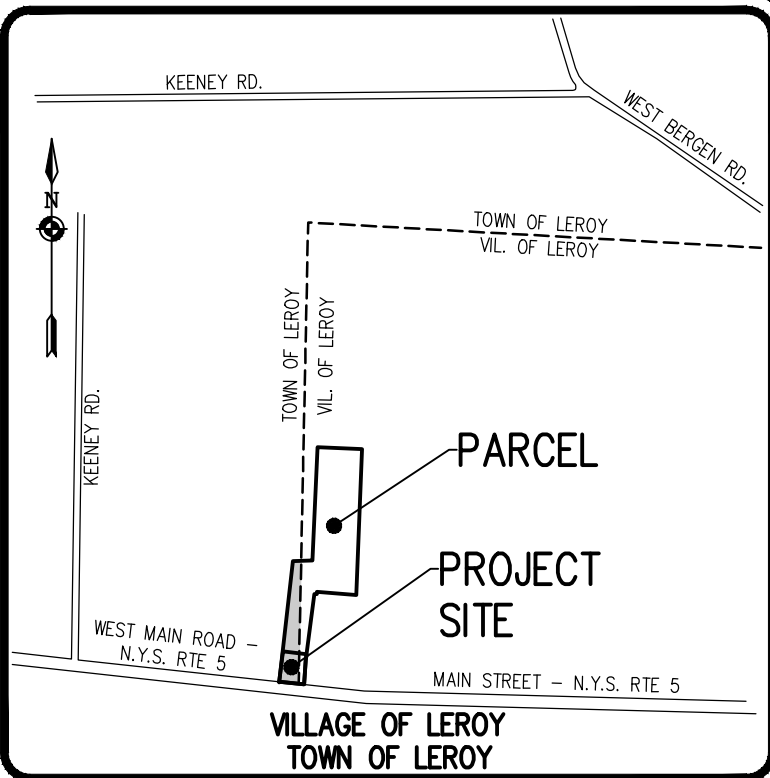
- BOUNDARY LINE
○ PROPERTY MARKER FOUND
- - - CENTERLINE

WEST MAIN ROAD – WEST MAIN
STREET – NEW YORK STATE ROUTE 5
(99' RIGHT-OF-WAY)

GRAPHIC SCALE



(IN FEET)
1 inch = 20 ft.



LOCATION MAP
NOT TO SCALE

Drawing Alteration
The following is an excerpt from the
New York State Education Law Article
145 Section 7209 and applies to this
drawing:
"It is a violation of this law for any
person, unless he is acting under the
direction of a licensed professional
engineer or land surveyor to alter any
item in any way. If an item bearing the
seal of an engineer or land surveyor is
altered, the altering engineer or land
surveyor shall affix to the item his seal
and the notation "altered by" followed
by his signature and the date of such
alteration, and a specific description
of the alteration."

REVISIONS	DATE	BY
1		
2		
3		
4		
5		
6		
7		

BME ASSOCIATES
ENGINEERS • SURVEYORS • LANDSCAPE ARCHITECTS
800 PARKER HILL DRIVE, SUITE 300
ROCHESTER, NEW YORK 14625
WWW.BMEPC.COM
585-377-7360



LEROI TACO BELL
PROJECT: LEROI TACO BELL
LOCATION: TOWN/VILLAGE OF LEROY, GENESSEE COUNTY, NEW YORK STATE
CLIENT: LEROI TACO BELL
1000 HOSPITALITY GROUP
1000 HOSPITALITY GROUP
1000 HOSPITALITY GROUP
ROCHESTER, NEW YORK, 14625

TURNING TEMPLATE EXHIBIT

DRAWING TITLE

PROJECT MANAGER
P. VARS
PROJECT ENGINEER
R. SPURR
DRAWN BY
N. PINTI
SCALE
1" = 20'
DATE ISSUED
AUGUST 6, 2026
PROJECT NO.

3036

DRAWING NO.
EXHIBIT

NOT APPROVED

This plan has not received final
approval of all reviewing agencies.
This plan is subject to revisions
until all approvals are obtained and
should not be used for construction
purposes.

Appendix E

Sanitary Sewer Pump Station

D3600-SERIES

Duplex Grinder Package

Liberty Pumps®

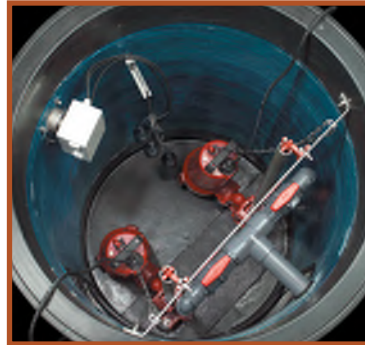
A Family and Employee Owned Company

Factory Assembled 2 hp Grinder Pumps 36" Diameter System 2" Discharge

Features

- Integral check valves
- QuickTree® pre-mounted float switch system for pump control and alarm
- 4" Inlet hub with rubber pipe seal
- Factory pre-assembled guide rail system
- Galvanized guide rail piping (stainless optional)
- Factory pre-assembled Schedule 80 PVC discharge piping with ball valves
- Fiberglass basin with anti-flotation collar
- Standard green fiberglass cover, solid (optional black steel, solid)
- All stainless steel supports and brackets pre-mounted
- NEMA 4X junction box for electrical connections, pre-mounted
- Stainless steel chain for pump lift-out

*Other custom basin sizes available, consult factory



4" Inlet
(unmounted)



Available with
LSG or LSGX-Series



innovate. evolve.

D3600-Series

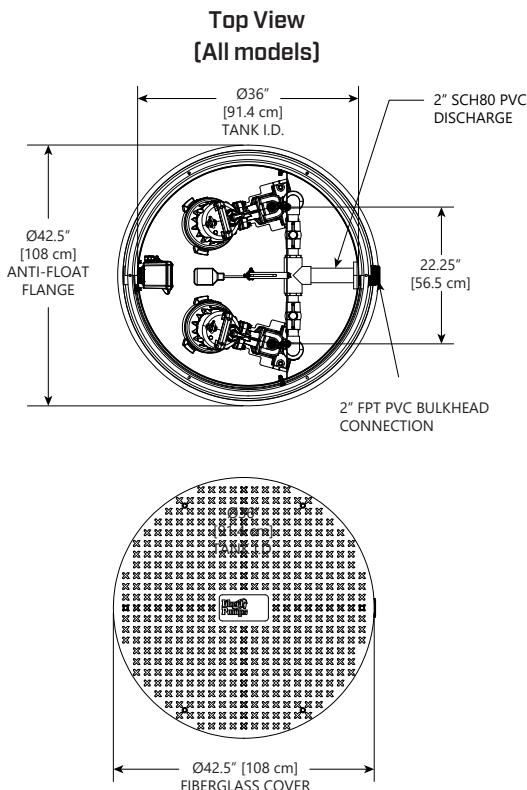
Features

- 2 hp Grinder pumps
- Choose from single-stage LSG or 2-stage LSGX-Series pumps
- Available in a wide range of voltages
- 2" Discharge
- 25' Power cable length for standard systems (35' power cable length for "-C" models)

(See LSG or LSGX-Series literature for complete pump specifications.)

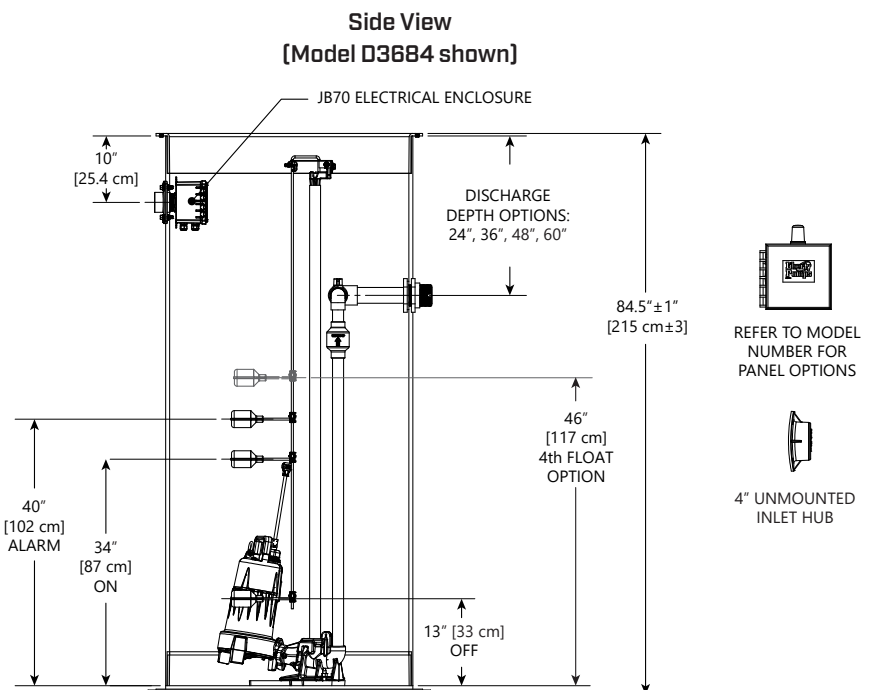
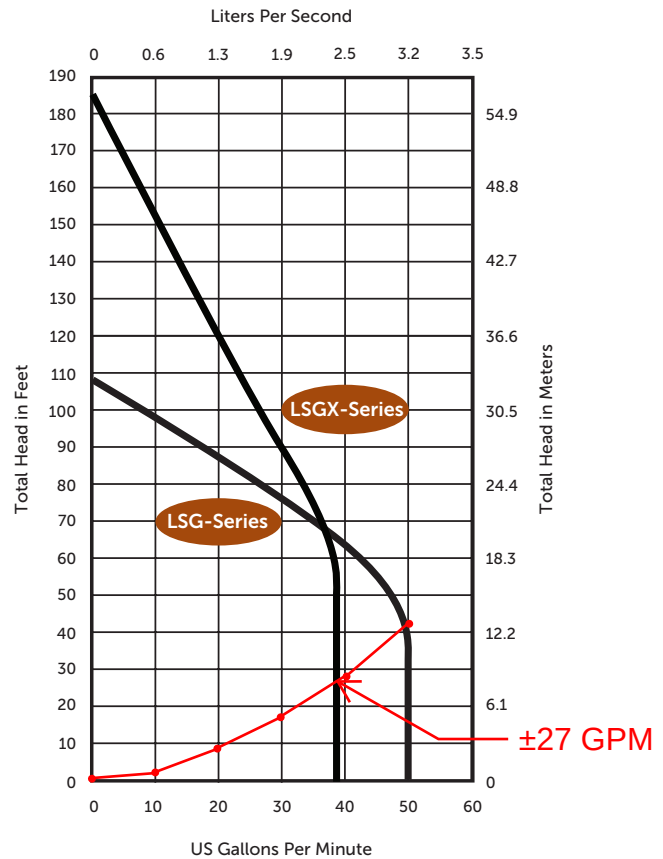


Dimensional Data



Performance Curves

60 Hz, 3450 RPM



Galvanized guide rails standard. For stainless steel rails add "S" to model number. Example: (steel) D3684LSG202-24-S

For steel cover option add "-SC" suffix to model number. Example: (steel) D3684LSG202-24-SC

Standard system shipped with three float configurations. For systems with the four-float option add a "-4F" suffix to the part number. Example: D3684LSG203-24-4F

Choice of Panel... AE-Series or IP-Series



AE24H=3
Control Panel
with Alarm
Included



AE-Series Panels [Standard]

Provides reliable alternating operation.

Features

- HOA switch offer manual operation of pumps (on circuit board)
- NEMA 4X enclosure for outdoor use
- Circuit breakers provide pump disconnect and branch circuit protection
- Alarm beacon and horn provide audio/visual warning of alarm condition
- Pump run indicator lights
- Auxiliary contacts
- Three level control float switches

For the complete AE-Series specifications, see AE-Series literature sheet.

IP-Series Panels [Optional]

IP-Series control panels incorporate the latest programmable pump features through a simple, easy-to-use touchpad on the inner door. Now pump programming and system monitoring is easier than ever.

Features

- NEMA 4X weatherproof enclosure for indoor or outdoor mounting
- Floatless level sensor is compact and eliminates multiple wide-angle float switches for controlling pump
- Redundant "ALARM" float switch included for added security
- Easy-to-use touchpad for programming pump on/off levels and alarm level in inches or centimeters
- Inner panel door provides increased safety
- Separate control and alarm fuses
- Digital display board for system monitoring
- Alarm beacon and horn provide audio/visual warning of alarm condition
- Test/Normal/Silence switch
- Auxiliary contacts
- Panel is field convertible for either demand dose or time dose applications

For the complete IP-Series specifications, see IP-Series literature sheet.

NOTE: To order the IP-Series panel option, add "-IP" suffix to the system model number. Example: D3648LSG202-IP



Floatless
Level Sensor

D3600-Series

36" diameter fiberglass basin available in heights of 48", 60", 72", 84", 96", and 120".

Systems listed below are shipped with a standard AE-Series control panel. To upgrade to an IP-Series panel add "-IP" suffix to the system model number. Example: D3672LSG202-24-IP See AE-Series or IP-Series literature for complete specifications.

MODEL	VOLTS	PHASE	WEIGHT	STANDARD PANEL AE-SERIES	OPTIONAL PANEL IP-SERIES
D3648 – 36" x 48" Basin (Discharge depth is 12" from top of basin)					
D3648LSG202	208/230	1	470 lbs	AE24H=3	IPD-24H
D3648LSG202-C	208/230	1	470 lbs	AE24HC=3-3	IPD-24HC
D3648LSG203	208/230	3	479 lbs	AE34=3-511	IPD-34-511
D3648LSG204	440–480	3	479 lbs	AE34=3-171	IPD-34-171
D3648LSG205	575	3	479 lbs	AE54=3-161	IPD-54-161
D3648LSGX202	208–230	1	482 lbs	AE24H=3	IPD-24H
D3648LSGX202-C	208–230	1	482 lbs	AE24HC=3-3	IPD-24HC
D3648LSGX203	208/230	3	491 lbs	AE34=3-511	IPD-34-511
D3648LSGX204	440–480	3	491 lbs	AE34=3-171	IPD-34-171
D3648LSGX205	575	3	491 lbs	AE54=3-161	IPD-54-161
D3660 – 36" x 60" Basin (Discharge depth is available in 24" or 36" from top of basin. Add -24 or -36 to model when ordering)					
D3660LSG202 (-24 or -36)	208/230	1	490 lbs	AE24H=3	IPD-24H
D3660LSG202-C (-24 or -36)	208/230	1	490 lbs	AE24HC=3-3	IPD-24HC
D3660LSG203 (-24 or -36)	208/230	3	499 lbs	AE34=3-511	IPD-34-511
D3660LSG204 (-24 or -36)	440–480	3	499 lbs	AE34=3-171	IPD-34-171
D3660LSG205 (-24 or -36)	575	3	499 lbs	AE54=3-161	IPD-54-161
D3660LSGX202 (-24 or -36)	208–230	1	502 lbs	AE24H=3	IPD-24H
D3660LSGX202-C (-24 or -36)	208–230	1	502 lbs	AE24HC=3-3	IPD-24HC
D3660LSGX203 (-24 or -36)	208/230	3	511 lbs	AE34=3-511	IPD-34-511
D3660LSGX204 (-24 or -36)	440–480	3	511 lbs	AE34=3-171	IPD-34-171
D3660LSGX205 (-24 or -36)	575	3	511 lbs	AE54=3-161	IPD-54-161
D3672 – 36" x 72" Basin (Discharge depth is available in 24", 36", or 48" from top of basin. Add -24, -36, or -48 to model when ordering)					
D3672LSG202 (-24, -36, or -48)	208/230	1	515 lbs	AE24H=3	IPD-24H
D3672LSG202-C (-24, -36, or -48)	208/230	1	515 lbs	AE24HC=3-3	IPD-24HC
D3672LSG203 (-24, -36, or -48)	208/230	3	524 lbs	AE34=3-511	IPD-34-511
D3672LSG204 (-24, -36, or -48)	440–480	3	524 lbs	AE34=3-171	IPD-34-171
D3672LSG205 (-24, -36, or -48)	575	3	524 lbs	AE54=3-161	IPD-54-161
D3672LSGX202 (-24, -36, or -48)	208–230	1	527 lbs	AE24H=3	IPD-24H
D3672LSGX202-C (-24, -36, or -48)	208–230	1	527 lbs	AE24HC=3-3	IPD-24HC
D3672LSGX203 (-24, -36, or -48)	208/230	3	536 lbs	AE34=3-511	IPD-34-511
D3672LSGX204 (-24, -36, or -48)	440–480	3	536 lbs	AE34=3-171	IPD-34-171
D3672LSGX205 (-24, -36, or -48)	575	3	536 lbs	AE54=3-161	IPD-54-161
D3684 – 36" x 84" Basin (Discharge depth is available in 24", 36", 48", or 60" from top of basin. Add -24, -36, -48, or -60 to model when ordering)					
D3684LSG202 (-24, -36, -48, or -60)	208/230	1	540 lbs	AE24H=3	IPD-24H
D3684LSG202-C (-24, -36, -48, or -60)	208/230	1	540 lbs	AE24HC=3-3	IPD-24HC
D3684LSG203 (-24, -36, -48, or -60)	208/230	3	549 lbs	AE34=3-511	IPD-34-511
D3684LSG204 (-24, -36, -48, or -60)	440–480	3	549 lbs	AE34=3-171	IPD-34-171
D3684LSG205 (-24, -36, -48, or -60)	575	3	549 lbs	AE54=3-161	IPD-54-161
D3684LSGX202 (-24, -36, -48, or -60)	208–230	1	552 lbs	AE24H=3	IPD-24H
D3684LSGX202-C (-24, -36, -48, or -60)	208–230	1	552 lbs	AE24HC=3-3	IPD-24HC
D3684LSGX203 (-24, -36, -48, or -60)	208/230	3	561 lbs	AE34=3-511	IPD-34-511
D3684LSGX204 (-24, -36, -48, or -60)	440–480	3	561 lbs	AE34=3-171	IPD-34-171
D3684LSGX205 (-24, -36, -48, or -60)	575	3	561 lbs	AE54=3-161	IPD-54-161
D3696 – 36" x 96" Basin (Discharge depth is available in 24", 36", 48", 60", or 72" from top of basin. Add -24, -36, -48, -60, or -72 to model when ordering)					
D3696LSG202 (-24, -36, -48, -60 or -72)	208/230	1	565 lbs	AE24H=3	IPD-24H
D3696LSG202-C (-24, -36, -48, -60 or -72)	208/230	1	565 lbs	AE24HC=3-3	IPD-24HC
D3696LSG203 (-24, -36, -48, -60 or -72)	208/230	3	574 lbs	AE34=3-511	IPD-34-511
D3696LSG204 (-24, -36, -48, -60 or -72)	440–480	3	574 lbs	AE34=3-171	IPD-34-171
D3696LSG205 (-24, -36, -48, -60 or -72)	575	3	574 lbs	AE54=3-161	IPD-54-161
D3696LSGX202 (-24, -36, -48, -60 or -72)	208–230	1	577 lbs	AE24H=3	IPD-24H
D3696LSGX202-C (-24, -36, -48, -60 or -72)	208–230	1	577 lbs	AE24HC=3-3	IPD-24HC
D3696LSGX203 (-24, -36, -48, -60 or -72)	208/230	3	586 lbs	AE34=3-511	IPD-34-511
D3696LSGX204 (-24, -36, -48, -60 or -72)	440–480	3	586 lbs	AE34=3-171	IPD-34-171
D3696LSGX205 (-24, -36, -48, -60 or -72)	575	3	586 lbs	AE54=3-161	IPD-54-161
D36120 – 36" x 120" Basin (Discharge depth is available in 24", 36", 48", 60", or 72" from top of basin. Add -24, -36, -48, -60, or -72 to model when ordering)					
D36120LSG202 (-24, -36, -48, -60 or -72)	208/230	1	632 lbs	AE24H=3	IPD-24H
D36120LSG202-C (-24, -36, -48, -60 or -72)	208/230	1	632 lbs	AE24HC=3-3	IPD-24HC
D36120LSG203 (-24, -36, -48, -60 or -72)	208/230	3	641 lbs	AE34=3-511	IPD-34-511
D36120LSG204 (-24, -36, -48, -60 or -72)	440–480	3	641 lbs	AE34=3-171	IPD-34-171
D36120LSG205 (-24, -36, -48, -60 or -72)	575	3	641 lbs	AE54=3-161	IPD-54-161
D36120LSGX202 (-24, -36, -48, -60 or -72)	208–230	1	644 lbs	AE24H=3	IPD-24H
D36120LSGX202-C (-24, -36, -48, -60 or -72)	208–230	1	644 lbs	AE24HC=3-3	IPD-24HC
D36120LSGX203 (-24, -36, -48, -60 or -72)	208/230	3	653 lbs	AE34=3-511	IPD-34-511
D36120LSGX204 (-24, -36, -48, -60 or -72)	440–480	3	653 lbs	AE34=3-171	IPD-34-171
D36120LSGX205 (-24, -36, -48, -60 or -72)	575	3	653 lbs	AE54=3-161	IPD-54-161

Specifications subject to change without notice.

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Phone 800-543-2550 - Fax 585-494-1839 - www.LibertyPumps.com